WIND	DESIGN

ASCE 7 -16 Risk Category **Directionality (Kd)** Mean Roof Ht (h) Parapet ht above grd Exposure Category Enclosure Classif. Internal pressure a =

0.85 14.5

0

0.18 3.0 ft

0.00 ft

Hip

26.6 deg

Enclosed Building

Minimum parapet height at building perimeter = **Roof Angle**

Type of roof

COMPONENTS & CLADDING

ULTIMATE LOADS:

	Basic Wind speed		139.0 mph (L	.RFD)				
	Base pressure (qh) =		35.7 psf (L	.RFD)				
Roof		Surface	e Pressure (psf) - LR	RFD Values				
Area	10 sf	20 sf	50 sf	100 sf	200 sf			
Negative Zone 1	-38.5 psf	-37.5 psf	-36.0 psf	-35.0 psf	-35.0 psf			
Negative Zone 2	-65.1 psf	-61.7 psf	-54.6 psf	-49.3 psf	-49.3 psf			
Negative Zone 3	-99.2 psf	-92.8 psf	-84.2 psf	-77.8 psf	-77.8 psf			
Positive All Zones	24.3 psf	22.1 psf	19.3 psf	17.1 psf	17.1 psf			
Overhang Zone 2	-78.5 psf	-78.5 psf	-78.5 psf	-78.5 psf	-78.5 psf			
Overhang Zone 3	-132.1 psf	-119.2 psf	-102.1 psf	-89.2 psf	-89.2 psf			
Walls	Surface Pressure (psf) - LRFD Values							
Area	10 sf	20 sf	50 sf	100 sf	500 sf			
Negative Zone 4	-45.7 psf	-43.8 psf	-41.3 psf	-39.4 psf	-35.0 psf			
Negative Zone 5	-56.4 psf	-52.6 psf	-47.6 psf	-43.8 psf	-35.0 psf			
Positive Zone 4 & 5	42.1 psf	40.2 psf	37.7 psf	35.8 psf	31.4 psf			
Parapets	Γ		Surface Pressur	re (psf) - LRFD Values				
Area		10 sf	50 sf	100 sf	500 sf			
Positive Interior (7one 4)		0.0 psf	0.0 psf	0.0 psf	0.0 psf			
Positive Corner (Zone 5)		0.0 psf	0.0 psf	0.0 psf	0.0 psf			
Negative Interior (Zone 4)		0.0 psf	0.0 psf	0.0 psf	0.0 psf			
Negative Corner (Zone 5)		0.0 psf	0.0 psf	0.0 psf	0.0 psf			

SERVICE LOADS:

	Basic Wind speed		107.7 mph (A	ASD)	
	Base pressure (qh) =		21.4 psf (A	ASD)	
Roof		(ASD)Su	rface Pressure (psf)	-ASD Values	
Area	10 sf	20 sf	50 sf	100 sf	200 sf
Negative Zone 1	-23.1 psf	-22.5 psf	-21.6 psf	-21.0 psf	-21.0 psf
Negative Zone 2	-40.3 psf	-37.0 psf	-32.8 psf	-29.6 psf	-29.6 psf
Negative Zone 3	-59.5 psf	-55.7 psf	-50.5 psf	-46.7 psf	-46.7 psf
Positive All Zones	14.6 psf	13.3 psf	11.6 psf	10.3 psf	10.3 psf
Overhang Zone 2	-47.1 psf	-47.1 psf	-47.1 psf	-47.1 psf	-47.1 psf
Overhang Zone 3	-79.2 psf	-71.5 psf	-61.3 psf	-53.5 psf	-53.5 psf
Walls		(ASD)Su	face Pressure (psf)	-ASD Values	
Area	10 sf	20 sf	50 sf	100 sf	500 sf
Negative Zone 4	-27.4 psf	-26.3 psf	-24.8 psf	-23.6 psf	-21.0 psf
Negative Zone 5	-33.8 psf	-31.6 psf	-28.5 psf	-26.3 psf	-21.0 psf
Positive Zone 4 & 5	25.3 psf	24.1 psf	22.6 psf	21.5 psf	18.8 psf
Parapets			(ASD)Surface Pres	ssure (psf) -ASD Values	
Area		10 sf	50 sf	100 sf	500 sf
Positive Interior (Zone 4)		0.0 psf	0.0 psf	0.0 psf	0.0 psf
Positive Corner (Zone 5)		0.0 psf	0.0 psf	0.0 psf	0.0 psf
Negative Interior (Zone 4)		0.0 psf	0.0 psf	0.0 psf	0.0 psf
Negative Corner (Zone 5)		0.0 psf	0.0 psf	0.0 psf	0.0 psf
				- I	

Note: Pressures listed above are in both values, Ultimate (LRFD) & Service or Nominal (ASD) which have been obtained by multiplying Ultimate values by 0.6. Use service values (ASD) for Wind Resistance Testing Compliance per FBC 1609.1.5

COMPONENTS & CLADDING PRESSURES - CANOPY



CANOPY ROOF PLAN



COMPONENTS & CLADDING ZONES - PLAN VIEW & WALL ELEVATION

DRAWN BY: CHECKED BY: PJ

SD-0.0

AS NOTED SCALE:

PARTIAL ROOF **TRUSS PLAN**



NOTES:

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787 03/30/2023 DATE: 23-0004 PROJECT NO .: DATE REVISION GENERAL REV. 5/3/2023

SHOP DWGS FOR **EXTERIOR ARCADE**

PROJECT:

2023.03.30











TRUSS TEMPORARY & PERMANENT BRACING GUIDELINES

DRAWN BY: CHECKED BY:

SCALE:

JM PJ

AS NOTED

SD-0.1

PARTIAL ROOF **TRUSS PLAN**

PROFESSIONAL ENGINEER SEAL STATE OF FLORIDA



NOTES:

REVISION GENERAL REV.

WINTER GARDEN, FL 34787 03/30/2023 23-0004 DATE 5/3/2023

SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC,

PROJECT:

DATE:

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2023.03.30

TOP CHORD CONTINUOUS LATERAL BRACING. REFER TO DETAIL C-2/TD1.

✓ REFER TO FIGURE 1-2/TD1.

DETAIL A CONNECTION OF DBR TO WEB NEAR CHORD DIAGONAL BRACING 1" MÀX TRUSS CHORD DETAIL B CONNECTION OF CLB TO WEB • DETAIL C CONNECTION OF CLB TO CHORDS CONT TOP - CONT BOTTOM CHORD BRACE



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CONNECTION	SECTION	LENGTH	AXIAL	INT.	FASTENER	PA	REQ.
#1	358CSJ18	10.46	3.15C	1.00	#10 SD	0.000	0
#2	358CSJ18	11.42	1.49C	0.70	#10 SD	0.000	0
#3	358CSJ18	3.83	0.80T	0.91	#10 SD	0.534	8
#4	358CSJ18	4.66	0.05T	0.03	#10 SD	0.177	8
#5	358CSJ18	2.33	0.64C	0.32	#10 SD	0.177	8
#6	358CSJ18	4.33	0.70C	0.29	#10 SD	0.177	8
#7	6CSJ18	0.53	1.33T	0.85	#10 SD	0.177	8

CONNECTION	SIMPSON	EACH	LOAD	UPLIFT/SHR	FASTENER	PA	REQ.
TRUSS CHORD	SEE NOTES	1	1.08		#10 SD	0.522	8
STEEL STUD	SEE NOTES			1.08	#10 SD	0.522	8
TRUSS CHORD	SEE NOTES	2	1.79		#10 SD	0.522	8
TRUSS GIRDER	SEE NOTES			1.79	#10 SD	0.522	8

PER AISI \$100-200)7		ACTUAL			ALLOWABLE			
MEMBER	SECTION	Ро	Vo	Мо	Ра	Va	Ма	RATIO	
BOTTOM CHORD	1-358CSJ18	3.15C	0.07	1.50	3.45	4.40	17.66	1.00	ASCE 7-16: PASSED
TOP CHORD	1-358CSJ18	1.49T	2.10	4.49	12.63	3.37	14.01	0.70	DESIGN METHOD - (ASD)
WEB	1-358CSJ18	0.04T	0.00	3.89	16.65	2.83	27.75	0.91	COMPONENT WIND PRESSURE DESIGN (END)

GENERAL NOTES

1. TRUSSES REQUIRE LATERAL BRACING. SEE TRUSS LAYOUT AND DETAIL SHEETS.

2. NUMBER OF FASTENERS NOTED IN CHART INSTALLED ON EACH END OF WEB. 3. ALLOWABLE FASTENER VALUES BASED ON COLD-FORMED STEEL DESIGN FORMULAE.

4. (ws) DENOTES WEB STIFFENER REQUIRED AT SUPPORT.

5. MEMBER DESIGN BASED ON SECTIONS IN UNITED_SSMA-7-8-2016 LIBRARY.

MAXIMUM DEFLECTIONS				
VERTICAL	0.137 in (L/128	4)		
HORIZONTAL	0.025 in	.,		
TOP OVERHANG	0.019 in (L/625)		
VERTICAL	0.040 in (L/439	, 8) IDEAD LOAD ONLYI		
VERTICAL	0.062 in (L/283	7) [LIVE LOAD ONLY]		
SUPPORT REACTIONS	DOWN	UPLIFT	HORIZONTAL	BEARING
LEFT	0.71 {0.71}	-1.08 [-2.09]	0.02	8.49
RIGHT	1.26 {1.26}	-1.79 [-3.47]	0.00	2.00
*UPLIFT LOAD COMBINAT	ION (TRUSS TO SUPI	PORT CONNECTION ONI	LY): 0.6 DEAD + 0.6 W	IND
{ } DENOTES 'DEAD	+LIVE ONLY'			
[] DENOTES 'WIND	ONLY' UPLIFT REAC	ΓΙΟΝ		
DESIGN DATA				
NUMBER OF TRUSS = 5 E	АСН			

NUMBER OF TRU	SS = 5 EACH	
PLATE STYLE	: OUT-OF-PLANE	
EAVE HEIGHT	: 12.00 ft (TOP OF WALL)	
BEARING	: 8.264 in	
SPACING	: 2.00 ft	
DEAD LOAD	: 10.00 PSF (TOP CHORD)	
DEAD LOAD	: 10.00 PSF (BOTTOM CHORD)	
LIVE LOAD	: 20.00 PSF (TOP CHORD)	
LIVE LOAD	: 0.00 PSF (BOTTOM CHORD)	
SNOW LOAD	: 0.00 PSF (GROUND) [Ct=1.00]	

WIND LOAD: 35.69 PSF (DESIGN)WIND SPEED: 139 MPH (EXPOSURE C)

OPEN CATEGORY : E TOPOGRAPHY (Kz) : 1 BUILDING CATEGORY : (2) GENERAL

DELETE WIND LOAD : RIGHT END SEISMIC COEFFICIENT: 0.0





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2023.03.30

PROJECT: SHOP DWGS FOR EXTERIOR ARCADE

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

 DATE:
 03/30/2023

 PROJECT NO.:
 23-0004

 REVISION
 DATE

 GENERAL REV.
 5/3/2023

NOTES:



TRUSS 1, 12, 13, 24 & 25

SCALE: AS NOTED

SD-1.0

DRAWN BY: CHECKED BY:



TRUSS CONNECTION KEY

SCALE: 1-1/2" = 1'-0"

(4) HEX HEAD GRABBERS #10x3/4" \bigcirc \bigcirc \bigcirc \bigcirc









XTERIOF	RARCAI
130 RUBY RED I	PLACE FC,
WINTER GARDE	N, FL 34787
DATE:	03/30/2023
PROJECT NO.:	23-0004

REVISION

NOTES:

PROJECT: **SHOP DWGS FOR** CADE

GENERAL REV. 5/3/2023

2023.03.30

DATE





CONNECTION	SECTION	LENGTH	AXIAL	INT.	FASTENER	PA	REQ.
#1	212CSJ18	10.46	3.15C	1.00	#10 SD	0.000	0
#2	212CSJ18	11.42	1.49C	0.70	#10 SD	0.000	0
#3	358CSJ18	3.83	0.80T	0.91	#10 SD	0.534	8
#4	212CSJ18	4.66	0.05T	0.03	#10 SD	0.177	8
#5	358CSJ18	2.33	0.64C	0.32	#10 SD	0.177	8
#6	358CSJ18	4.33	0.70C	0.29	#10 SD	0.177	8
#7	6CSJ18	0.53	1.33T	0.85	#10 SD	0.177	8

CONNECTION	SIMPSON	EACH	LOAD	UPLIFT/SHR	FASTENER	PA	REQ.
TRUSS CHORD	SEE NOTES	1	1.08		#10 SD	0.522	8
STEEL STUD	SEE NOTES			1.08	#10 SD	0.522	8
TRUSS CHORD	SEE NOTES	2	1.79		#10 SD	0.522	8
TRUSS GIRDER	SEE NOTES			1.79	#10 SD	0.522	8

PER AISI \$100-200)7		ACTUAL			ALLOWABLE			
MEMBER	SECTION	Ро	Vo	Мо	Ра	Va	Ма	RATIO	
BOTTOM CHORD	1-212CSJ18	3.15C	0.07	1.50	3.45	4.40	17.66	1.00	ASCE 7-16: PASSED
TOP CHORD	1-212CSJ18	1.49T	2.10	4.49	12.63	3.37	14.01	0.70	DESIGN METHOD - (ASD)
WEB	1-358CSJ18	0.04T	0.00	3.89	16.65	2.83	27.75	0.91	COMPONENT WIND PRESSURE DESIGN (END)

GENERAL NOTES

1. TRUSSES REQUIRE LATERAL BRACING. SEE TRUSS LAYOUT AND DETAIL SHEETS. 2. NUMBER OF FASTENERS NOTED IN CHART INSTALLED ON EACH END OF WEB.

3. ALLOWABLE FASTENER VALUES BASED ON COLD-FORMED STEEL DESIGN FORMULAE. 4. (ws) DENOTES WEB STIFFENER REQUIRED AT SUPPORT. 5. MEMBER DESIGN BASED ON SECTIONS IN UNITED_SSMA-7-8-2016 LIBRARY.

MAXIMUM DEFLECTIONS				
VERTICAL	0.137 in (L/128	4)		
HORIZONTAL	0.025 in			
TOP OVERHANG	0.019 in (L/625)		
VERTICAL	0.040 in (L/439	8) [DEAD LOAD ONLY]		
VERTICAL	0.062 in (L/283	7) [LIVE LOAD ONLY]		
SUPPORT REACTIONS	DOWN	UPLIFT	HORIZONTAL	BEARING
LEFT	0.71 {0.71}	-1.08 [-2.09]	0.02	8.49
RIGHT	1.26 {1.26}	-1.79 [-3.47]	0.00	2.00
*UPLIFT LOAD COMBINATI	ON (TRUSS TO SUPI	PORT CONNECTION ONI	_Y): 0.6 DEAD + 0.6 W	IND
{ } DENOTES 'DEAD	+LIVE ONLY'			
[] DENOTES 'WIND	ONLY' UPLIFT REAC	ΓΙΟΝ		
DESIGN DATA				
	VOU			

NUMBER OF TRUSS =	E 2 EACH
PLATE STYLE	: OUT-OF-PLANE
EAVE HEIGHT	: 12.00 ft (TOP OF WALL)
BEARING	: 8.264 in
SPACING	: 2.00 ft
DEAD LOAD	: 10.00 PSF (TOP CHORD)
DEAD LOAD	: 10.00 PSF (BOTTOM CHORD)
LIVE LOAD	: 20.00 PSF (TOP CHORD)
LIVE LOAD	: 0.00 PSF (BOTTOM CHORD)
SNOW LOAD	: 0.00 PSF (GROUND) [Ct=1.00]
WIND LOAD	: 35.69 PSF (DESIGN)
WIND SPEED	: 139 MPH (EXPOSURE C)
OPEN CATEGORY	:E
TOPOGRAPHY (Kz)	:1
BUILDING CATEGORY	(: (2) GENERAL
DELETE WIND LOAD	: RIGHT END
SEISMIC COEFFICIEN	IT: 0.0







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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

DATE: 03/30/2023 PROJECT NO.: 23-0004 DATE REVISION GENERAL REV. 5/3/2023

NOTES:



TRUSS 2 & 11

SCALE: AS NOTED

SD-2.0

DRAWN BY: CHECKED BY:



TRUSS CONNECTION KEY

SCALE: 1-1/2" = 1'-0"











SCALE: 6" = 1'-0"

SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

GENERAL REV. 5/3/2023

PROJECT:

DATE:

PROJECT NO .:

REVISION

NOTES:

2023.03.30

03/30/2023

23-0004

DATE

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CONNECTION	SECTION	LENGTH	AXIAL	INT.	FASTENER	PA	REQ.
#1	212CSJ18	10.46	2.87C	0.93	#10 SD	0.000	0
#2	212CSJ18	11.42	1.25C	0.76	#10 SD	0.000	0
#3	358CSJ18	3.83	0.60T	0.93	#10 SD	0.534	8
#4	212CSJ18	4.66	0.03T	0.03	#10 SD	0.177	8
#5	6CSJ18	0.53	1.39T	0.79	#10 SD	0.177	8

CONNECTION	SIMPSON	EACH	LOAD	UPLIFT/SHR	FASTENER	PA	REQ.
TRUSS CHORD	SEE NOTES	1	0.86		#10 SD	0.522	8
STEEL STUD	SEE NOTES			1.03	#10 SD	0.522	8
TRUSS CHORD	SEE NOTES	2	1.83		#10 SD	0.522	8
TRUSS GIRDER	SEE NOTES			1.83	#10 SD	0.522	8

PER AISI \$100-200)/		ACIUAL			ALLOWABLE			
MEMBER	SECTION	Ро	Vo	Мо	Ра	Va	Ма	RATIO	
BOTTOM CHORD	1-212CSJ18	2.87C	0.09	1.63	3.45	4.48	16.39	1.00	ASCE 7-16: PASSED
TOP CHORD	1-212CSJ18	1.25T	1.90	4.35	12.45	3.21	13.97	0.70	DESIGN METHOD - (ASD)
WEB	1-358CSJ18	0.04T	0.00	3.67	16.57	2.69	28.01	0.91	COMPONENT WIND PRESSURE DESIGN (END)

GENERAL NOTES

TRUSSES REQUIRE LATERAL BRACING. SEE TRUSS LAYOUT AND DETAIL SHEETS.
 NUMBER OF FASTENERS NOTED IN CHART INSTALLED ON EACH END OF WEB.
 ALLOWABLE FASTENER VALUES BASED ON COLD-FORMED STEEL DESIGN FORMULAE.
 (ws) DENOTES WEB STIFFENER REQUIRED AT SUPPORT.

5. MEMBER DESIGN BASED ON SECTIONS IN UNITED_SSMA-7-8-2016 LIBRARY.

MAXIN	IUM DEFLECTIONS				
	VERTICAL	0.142 in (L/1284)			
	HORIZONTAL	0.031 in			
	TOP OVERHANG	0.021 in (L/625)			
	VERTICAL	0.042 in (L/4398) [D	EAD LOAD ONLY		
	VERTICAL	0.067 in (L/2837) [L	IVE LOAD ONLY]		
SUPPO	ORT REACTIONS	DOWN	UPLIFT	HORIZONTAI	BFARING
	LEFT	0.73 {0.73}	-1.03 [-2.02]	0.04	9.53
	RIGHT	1.32 {1.32}	-1.83 [-3.64]	0.00	2.00
*UPLI	T LOAD COMBINATIO	N (TRUSS TO SUPPORT	CONNECTION ONLY):	0.6 DEAD + 0.6 WIND	
{	} DENOTES 'DEAD+L	IVE ONLY'			
i -] DENOTES 'WIND OF	NLY' UPLIFT REACTION			
DESIG	N DATA				
NUMB	FR OF TRUSS = $2 FAC$	H			
PI ATF	STYLE : OUT-	OF-PLANE			

SCALE: 1-1/2" = 1'-0"

FLAIL STILL	. OUT-OF-FLANL
EAVE HEIGHT	: 12.00 ft (TOP OF WALL)
BEARING	: 9.523 in
SPACING	: 2.00 ft
DEAD LOAD	: 10.00 PSF (TOP CHORD)
DEAD LOAD	: 10.00 PSF (BOTTOM CHORD)
LIVE LOAD	: 20.00 PSF (TOP CHORD)
LIVE LOAD	: 0.00 PSF (BOTTOM CHORD)
SNOW LOAD	: 0.00 PSF (GROUND) [Ct=1.00]
WIND LOAD	: 35.69 PSF (DESIGN)
WIND SPEED	: 139 MPH (EXPOSURE C)
OPEN CATEGORY	:E
TOPOGRAPHY (Kz)	:1
BUILDING CATEGORY	: (2) GENERAL
DELETE WIND LOAD	: RIGHT END
SEISMIC COEFFICIEN	IT: 0.0





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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

DATE: 03/30/2023 PROJECT NO.: 23-0004 DATE REVISION GENERAL REV. 5/3/2023

NOTES:



TRUSS 3 & 10

AS NOTED SCALE:

SD-3.0

DRAWN BY: CHECKED BY:









3





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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

DATE: PROJECT NO .: REVISION GENERAL REV. 5/3/2023

03/30/2023 23-0004 DATE

NOTES:



TRUSS 3 & 10 CONNECTIONS SCALE: AS NOTED

SD-3.1

DRAWN BY: CHECKED BY:

CONNECTION	SECTION	LENGTH	AXIAL	INT.	FASTENER	PA	REQ.
#1	212CSJ18	6.44	0.62C	0.82	#10 SD	0.000	0
#2	212CSJ18	7.83	0.47C	0.37	#10 SD	0.000	0
#3	358CSJ18	2.58	0.33T	0.05	#10 SD	0.347	8
#4	212CSJ18	4.75	0.57T	0.34	#10 SD	0.177	8
#5	6CSJ18	0.67	0.10C	0.03	#10 SD	0.177	8

CONNECTION	SIMPSON	EACH	LOAD	UPLIFT/SHR	FASTENER	PA	REQ.
CHORD WALL	SEE NOTES			0.33	#10 SD	0.263	8
TRUSS CHORD	SEE NOTES	1	0.33		#10 SD	0.263	8
STEEL STUD	SEE NOTES			0.33	#10 SD	0.263	8
TRUSS CHORD	SEE NOTES	1	0.39		#10 SD	0.263	8
HIP RAFTER	SEE NOTES			0.39	#10 SD	0.263	8

PER AISI S100-200)7		ACTUAL			ALLOWABLE			
MEMBER	SECTION	Ро	Vo	Мо	Ра	Va	Ма	RATIO	
BOTTOM CHORD	1-212CSJ18	0.62C	0.11	2.78	2.68	1.38	4.81	0.89	ASCE 7-16: PASSED
TOP CHORD	1-212CSJ18	0.47T	0.13	1.57	5.91	1.38	4.71	0.46	DESIGN METHOD - (ASD)
WEB	1-358CSJ18	0.57T	0.00	0.13	4.63	2.83	3.72	0.04	COMPONENT WIND PRESSURE DESIGN (END)

1. TRUSSES REQUIRE LATERAL BRACING. SEE TRUSS LAYOUT AND DETAIL SHEETS. 2. NUMBER OF FASTENERS NOTED IN CHART INSTALLED ON EACH END OF WEB.

2. NUMBER OF FASTENERS NOTED IN CHART INSTALLED ON EACH END OF WEB. 3. ALLOWABLE FASTENER VALUES BASED ON COLD-FORMED STEEL DESIGN FORMULAE.

4. (ws) DENOTES WEB STIFFENER REQUIRED AT SUPPORT.

5. MEMBER DESIGN BASED ON SECTIONS IN UNITED_SSMA-7-8-2016 LIBRARY.

MAXIMUM DEFLE	CTIONS			
VERTICAL	0.043 in (L/20	90)		
HORIZONT	AL 0.005 in			
TOP OVER	HANG 0.026 in (L/50	0)		
VERTICAL	0.022 in (L/35	44) [DEAD LOAD ONLY]		
VERTICAL	0.017 in (L/47	95) [LIVE LOAD ONLY]		
SUPPORT REACTI	ONS DOWN	UPLIFT	HORIZONTAL	BEARING
LEFT	0.36 {0.33}	-0.33 [-0.73]	0.32	6.00
RIGHT	0.26 {0.26}	-0.37 [-0.79]	0.00	2.00
*UPLIFT LOAD CO	MBINATION (TRUSS TO SU	PPORT CONNECTION ON	LY): 0.6 DEAD + 0.6 W	IND
{ } DENOTE	S 'DEAD+LIVE ONLY'			
[] DENOTES	5 'WIND ONLY' UPLIFT REA	CTION		
DESIGN DATA				
NUMBER OF TRUS	SS = 2 EACH			
PLATE STYLE	: OUT-OF-PLANE			
EAVE HEIGHT	: 12.00 ft (TOP OF WAL	.L)		
BEARING	: 6.000 in			
SDACING	· 2 00 ft			

SPACING: 2.00 ftDEAD LOAD: 10.00 PSF (TOP CHORD)DEAD LOAD: 10.00 PSF (BOTTOM CHORD)LIVE LOAD: 20.00 PSF (TOP CHORD)LIVE LOAD: 0.00 PSF (BOTTOM CHORD)SNOW LOAD: 0.00 PSF (GROUND) [Ct=1.00]WIND LOAD: 35.69 PSF (DESIGN)WIND SPEED: 139 MPH (EXPOSURE C)OPEN CATEGORY: ETOPOGRAPHY (Kz): 1

TOPOGRAPHY (Kz) : 1 BUILDING CATEGORY : (2) GENERAL DELETE WIND LOAD : RIGHT END SEISMIC COEFFICIENT: 0.0







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2023.03.30

PROJECT: SHOP DWGS FOR EXTERIOR ARCADE

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

 DATE:
 03/30/2023

 PROJECT NO.:
 23-0004

 REVISION
 DATE

 GENERAL REV.
 5/3/2023

NOTES:



TRUSS 4 & 9

SCALE: AS NOTED



DRAWN BY: CHECKED BY:







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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

03/30/2023 DATE: 23-0004 PROJECT NO .: DATE REVISION GENERAL REV. 5/3/2023

NOTES:



TRUSS 4 & 9 CONNECTIONS SCALE: AS NOTED

SD-4.1

DRAWN BY: CHECKED BY:

CONNECTION	SECTION	LENGTH	AXIAL	INT.	FASTENER	PA	REQ.
#1	212CSJ18	6.44	0.62C	0.82	#10 SD	0.000	0
#2	212CSJ18	7.83	0.47C	0.37	#10 SD	0.000	0
#3	358CSJ18	2.58	0.33T	0.05	#10 SD	0.347	8
#4	212CSJ18	4.75	0.57T	0.34	#10 SD	0.177	8
#5	6CSJ18	0.67	0.10C	0.03	#10 SD	0.177	8

CONNECTION	SIMPSON	EACH	LOAD	UPLIFT/SHR	FASTENER	PA	REQ.
CHORD WALL	SEE NOTES			0.33	#10 SD	0.263	8
TRUSS CHORD	SEE NOTES	1	0.33		#10 SD	0.263	8
STEEL STUD	SEE NOTES			0.33	#10 SD	0.263	8
TRUSS CHORD	SEE NOTES	1	0.39		#10 SD	0.263	8
HIP RAFTER	SEE NOTES			0.39	#10 SD	0.263	8

PER AISI S100-200)7		ACTUAL			ALLOWABLE			
MEMBER	SECTION	Ро	Vo	Мо	Ра	Va	Ма	RATIO	
BOTTOM CHORD	1-212CSJ18	0.62C	0.11	2.78	2.68	1.38	4.81	0.89	ASCE 7-16: PASSED
TOP CHORD	1-212CSJ18	0.47T	0.13	1.57	5.91	1.38	4.71	0.46	DESIGN METHOD - (ASD)
WEB	1-358CSJ18	0.57T	0.00	0.13	4.63	2.83	3.72	0.04	COMPONENT WIND PRESSURE DESIGN (END)

TRUSSES REQUIRE LATERAL BRACING. SEE TRUSS LAYOUT AND DETAIL SHEETS.
 NUMBER OF FASTENERS NOTED IN CHART INSTALLED ON EACH END OF WEB.
 ALLOWABLE FASTENER VALUES BASED ON COLD-FORMED STEEL DESIGN FORMULAE.
 (ws) DENOTES WEB STIFFENER REQUIRED AT SUPPORT.

5. MEMBER DESIGN BASED ON SECTIONS IN UNITED_SSMA-7-8-2016 LIBRARY.

MAXIMUM DEFLEC	TIONS			
VERTICAL	0.043 in (L/209	90)		
HORIZONTA	L 0.005 in			
TOP OVERH	ANG 0.026 in (L/500))		
VERTICAL	0.022 in (L/354	44) [DEAD LOAD ONLY]		
VERTICAL	0.017 in (L/479	95) [LIVE LOAD ONLY]		
				DEADING
				6 00
	0.30 (0.33)	-0.33 [-0.73]	0.52	0.00
	U.20 {U.20}			2.00
		PORTCONNECTION ON	LT). 0.0 DEAD + 0.0 W	
[]DENOTES	WIND UNLY UPLIFT REAU	TION		
DESIGN DATA				
NUMBER OF TRUS	S = 2 EACH			
PLATE STYLE	: OUT-OF-PLANE			
EAVE HEIGHT	: 12.00 ft (TOP OF WAL	L)		
BFARING	: 6.000 in			

NUMBER OF TRUSS =	2 EACH
PLATE STYLE	: OUT-OF-PLANE
EAVE HEIGHT	: 12.00 ft (TOP OF WALL)
BEARING	: 6.000 in
SPACING	: 2.00 ft
DEAD LOAD	: 10.00 PSF (TOP CHORD)
DEAD LOAD	: 10.00 PSF (BOTTOM CHORD)
LIVE LOAD	: 20.00 PSF (TOP CHORD)
LIVE LOAD	: 0.00 PSF (BOTTOM CHORD)
SNOW LOAD	: 0.00 PSF (GROUND) [Ct=1.00]
WIND LOAD	: 35.69 PSF (DESIGN)
WIND SPEED	: 139 MPH (EXPOSURE C)
OPEN CATEGORY	:E
TOPOGRAPHY (Kz)	:1
BUILDING CATEGORY	: (2) GENERAL

DELETE WIND LOAD : RIGHT END SEISMIC COEFFICIENT: 0.0







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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

DATE: 03/30/2023 PROJECT NO.: 23-0004 DATE REVISION GENERAL REV. 5/3/2023

NOTES:



TRUSS 5 & 8

SCALE: AS NOTED

SD-5.0

DRAWN BY: CHECKED BY:







1



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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

03/30/2023 DATE: 23-0004 PROJECT NO .: DATE REVISION GENERAL REV. 5/3/2023

NOTES:



TRUSS 5 & 8 CONNECTIONS SCALE: AS NOTED

SD-5.1

DRAWN BY: CHECKED BY:

CONNECTION	SECTION	LENGTH	AXIAL	INT.	FASTENER	PA	REQ.
#1	358CSJ18	6.44	0.62C	0.82	#10 SD	0.000	0
# 2	358CSJ18	7.83	0.47C	0.37	#10 SD	0.000	0
#3	358CSJ18	2.58	0.33T	0.05	#10 SD	0.347	8
#4	6CSJ18	4.75	0.57T	0.34	#10 SD	0.177	8

CONNECTION	SIMPSON	EACH	LOAD	UPLIFT/SHR	FASTENER	PA	REQ.
CHORD WALL	SEE NOTES			0.33	#10 SD	0.263	8
TRUSS CHORD	SEE NOTES	1	0.33		#10 SD	0.263	8
STEEL STUD	SEE NOTES			0.33	#10 SD	0.263	8
TRUSS CHORD	SEE NOTES	1	0.39		#10 SD	0.263	8
HIP RAFTER	SEE NOTES			0.39	#10 SD	0.263	8

PER AISI S100-200)7		ACTUAL			ALLOWABLE			
MEMBER	SECTION	Ро	Vo	Мо	Ра	Va	Ма	RATIO	
BOTTOM CHORD	1-358CSJ18	0.62C	0.11	2.78	2.68	1.38	4.81	0.89	ASCE 7-16: PASSED
TOP CHORD	1-358CSJ18	0.47T	0.13	1.57	5.91	1.38	4.71	0.46	DESIGN METHOD - (ASD)
WEB	1-358CSJ18	0.57T	0.00	0.13	4.63	2.83	3.72	0.04	COMPONENT WIND PRESSURE DESIGN (END)

1. TRUSSES REQUIRE LATERAL BRACING. SEE TRUSS LAYOUT AND DETAIL SHEETS.

2. NUMBER OF FASTENERS NOTED IN CHART INSTALLED ON EACH END OF WEB. 3. ALLOWABLE FASTENER VALUES BASED ON COLD-FORMED STEEL DESIGN FORMULAE.

4. (ws) DENOTES WEB STIFFENER REQUIRED AT SUPPORT. 5. MEMBER DESIGN BASED ON SECTIONS IN UNITED_SSMA-7-8-2016 LIBRARY.

MAXIMUM DEFLECTIONS	5			
VERTICAL	0.043 in (L/209	0)		
HORIZONTAL	0.005 in			
TOP OVERHANG	0.026 in (L/500)			
VERTICAL	0.022 in (L/354	4) [DEAD LOAD ONLY]		
VERTICAL	0.017 in (L/479	5) [LIVE LOAD ONLY]		
SUPPORT REACTIONS	DOWN	UPLIFT	HORIZONTAL	BEARING
LEFT	0.36 {0.33}	-0.33 [-0.73]	0.32	6.00
RIGHT	0.26 {0.26}	-0.37 [-0.79]	0.00	2.00
*UPLIFT LOAD COMBINA	TION (TRUSS TO SUPE	PORT CONNECTION ONI	LY): 0.6 DEAD + 0.6 W	IND
{ } DENOTES 'DEA	D+LIVE ONLY'		,	
I DENOTES 'WINI	D ONLY' UPLIFT REACT	ΓΙΟΝ		
1 1				
DESIGN DATA				
	EVCH			
PLAIE SITLE : C	JUI-UF-PLANE			
EAVE HEIGHI : 1	12.00π (IUP OF WALL)		
BEARING · F	5 000 in			

- BEARING SPACING DEAD LOAD DEAD LOAD LIVE LOAD LIVE LOAD : 6.000 in : 2.00 ft : 10.00 PSF (TOP CHORD) : 10.00 PSF (BOTTOM CHORD) : 20.00 PSF (TOP CHORD) : 0.00 PSF (BOTTOM CHORD)
- SNOW LOAD : 0.00 PSF (GROUND) [Ct=1.00]
- SNOW EOAD: 0.00131 (dk00kb) [ct=1.WIND LOAD: 35.69 PSF (DESIGN)WIND SPEED: 139 MPH (EXPOSURE C)OPEN CATEGORY: ETOPOGRAPHY (Kz): 1
- BUILDING CATEGORY : (2) GENERAL
- DELETE WIND LOAD : RIGHT END SEISMIC COEFFICIENT: 0.0







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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

DATE: 03/30/2023 PROJECT NO.: 23-0004 DATE REVISION GENERAL REV. 5/3/2023

NOTES:

_____ _____



TRUSS 6 & 7

AS NOTED SCALE:

SD-6.0

DRAWN BY: CHECKED BY:



TRUSS CONNECTION KEY

SCALE: 1-1/2" = 1'-0"



SCALE: 6" = 1'-0"



(4) HEX HEAD GRABBERS #10x3/4"





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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

03/30/2023 DATE: 23-0004 PROJECT NO .: DATE REVISION GENERAL REV. 5/3/2023

NOTES:

_____ _____

TRUSS 6 & 7 CONNECTIONS SCALE: AS NOTED

SD-6.1

DRAWN BY: CHECKED BY:

CONNECTION	SECTION	LE	NGTH	AXIAL	INT.	FASTENER	PA	REQ.
#1	6CSJ18	1	L 4.75	2.87C	0.93	#10 SD	0.000	0
#2	358CSJ18	1	L 5.8 3	1.25C	0.76	#10 SD	0.000	0
#3	358CSJ18	Ę	5.21	0.60T	0.93	#10 SD	0.534	8
#4	358CSJ18	Ę	5.00	0.03T	0.03	#10 SD	0.177	8
#5	358CSJ18	3	3.92	0.54C	0.39	#10 SD	0.177	8
#6	358CSJ18	4	4.58	0.86C	0.22	#10 SD	0.177	8
#7	358CSJ18	2	2.42	1.39T	0.79	#10 SD	0.177	8
#8	358CSJ18	3	3.75	0.86C	0.22	#10 SD	0.177	8
#9	358CSJ18	1	l.17	1.39T	0.79	#10 SD	0.177	8
#10	358CSJ18	().50	1.39T	0.79	#10 SD	0.177	8
CONNECTION	SIMPSON	EACH	LOAD	UPLIF	T/SHR	FASTENER	PA	REQ.
TRUSS CHORD	SEE NOTES	1	0.86			#10 SD	0.522	8
STEEL STUD	SEE NOTES			1	.03	#10 SD	0.522	8
TRUSS CHORD	SEE NOTES	2	1.83			#10 SD	0.522	8
TRUSS GIRDER	SEE NOTES			1	.83	#10 SD	0.522	8

PER AISI S100-200)7		ACTUAL			ALLOWABLE			
MEMBER	SECTION	Ро	Vo	Мо	Ра	Va	Ма	RATIO	
BOTTOM CHORD	1-6CSJ18	2.87C	0.09	1.63	3.45	4.48	16.39	1.00	ASCE 7-16: PASSED
TOP CHORD	1-358CSJ18	1.25T	1.90	4.35	12.45	3.21	13.97	0.70	DESIGN METHOD - (ASD)
WEB	1-358CSJ18	0.04T	0.00	3.67	16.57	2.69	28.01	0.91	COMPONENT WIND PRESSURE DESIGN (END)

1. TRUSSES REQUIRE LATERAL BRACING. SEE TRUSS LAYOUT AND DETAIL SHEETS.

2. NUMBER OF FASTENERS NOTED IN CHART INSTALLED ON EACH END OF WEB. 3. ALLOWABLE FASTENER VALUES BASED ON COLD-FORMED STEEL DESIGN FORMULAE.

4. (ws) DENOTES WEB STIFFENER REQUIRED AT SUPPORT.

5. MEMBER DESIGN BASED ON SECTIONS IN UNITED_SSMA-7-8-2016 LIBRARY.

MAXIMUM DEFLECTION VERTICAL HORIZONTAL TOP OVERHAN VERTICAL VERTICAL	ons Ng	0.142 in (L/128 0.031 in 0.021 in (L/625 0.042 in (L/439 0.067 in (L/283	34) 5) 98) [DEAD LOA 87) [LIVE LOAD	D ONLY] ONLY]		
SUPPORT REACTIONS LEFT RIGHT *UPLIFT LOAD COMB { } DENOTES 'V	S SINATIO DEAD+L VIND ON	DOWN 0.73 {0.73} 1.32 {1.32} N (TRUSS TO SUP IVE ONLY' NLY' UPLIFT REAC	UPLIFT -1.03 [- -1.83 [- PORT CONNE TION	2.02] 3.64] CTION ONLY):	HORIZONTAL 0.04 0.00 0.6 DEAD + 0.6 WIND	BEARING 9.53 2.00
DESIGN DATA						
NUMBER OF TRUSS PLATE STYLE EAVE HEIGHT BEARING SPACING DEAD LOAD DEAD LOAD LIVE LOAD LIVE LOAD SNOW LOAD WIND LOAD WIND SPEED OPEN CATEGORY TOPOGRAPHY (Kz) BUILDING CATEGORY DELETE WIND LOAD SEISMIC COEFFICIE	= 2 EAC : OUT- : 12.0 : 9.52 : 2.00 : 10.0 : 10.0 : 20.0 : 0.00 : 0.00 : 35.6 : 139 : E : 1 Y : (2) G : RIGH VT: 0.0	CH OF-PLANE O ft (TOP OF WALI 3 in ft O PSF (TOP CHOR O PSF (BOTTOM C O PSF (TOP CHOR PSF (BOTTOM CH PSF (GROUND) [I 9 PSF (DESIGN) MPH (EXPOSURE ENERAL IT END	L) 2D) 2D0 40RD) 40RD) Ct=1.00] C)			

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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

03/30/2023 23-0004 DATE: PROJECT NO .: DATE REVISION GENERAL REV. 5/3/2023

NOTES:

HIP TRUSS

AS NOTED SCALE:

SD-7.0

DRAWN BY: CHECKED BY:

CONNECTION	SECTION	LENGTH	AXIAL	INT.	FASTENER	PA	REQ.
#1	358CSJ18	10.46	3.15C	1.00	#10 SD	0.000	0
#2	358CSJ18	11.42	1.49C	0.70	#10 SD	0.000	0
#3	358CSJ18	3.83	0.80T	0.91	#10 SD	0.534	5
#4	358CSJ18	4.66	0.05T	0.03	#10 SD	0.177	4
#5	358CSJ18	2.33	0.64C	0.32	#10 SD	0.177	4
#6	358CSJ18	4.33	0.70C	0.29	#10 SD	0.177	4
#7	358CSJ18	0.53	1.33T	0.85	#10 SD	0.177	8

CONNECTION	SECTION	LENGTH	AXIAL	INT.	FASTENER	PA	REQ.
#1	6CSJ18	8.69	0.69C	0.91	#10 SD	0.000	0
# 2	358CSJ18	9.50	0.52C	0.55	#10 SD	0.000	0
#3	358CSJ18	3.25	0.38T	0.09	#10 SD	0.347	8
#4	358CSJ18	4.13	0.59T	0.43	#10 SD	0.177	8
#5	358CSJ18	1.92	0.11T	0.05	#10 SD	0.177	8
#6	358CSJ18	2.59	0.77C	0.53	#10 SD	0.177	8
#7	6CSJ18	0.67	0.32T	0.17	#10 SD	0.177	8

CONNECTION	SIMPSON	FACH			EASTENED	D٨	PEO
CONNECTION	51111 5014	LAUI	LUAD		TASTENEN		NLQ.
CHORD WALL	SEE NOTES			0.43	#10 SD	0.263	8
TRUSS CHORD	SEE NOTES	1	0.49		#10 SD	0.263	8
STEEL STUD	SEE NOTES			0.46	#10 SD	0.263	8
TRUSS CHORD	SEE NOTES	2	0.52		#10 SD	0.263	8
HIP GIRDER	SEE NOTES			0.50	#10 SD	0.263	8

PER AISI S100-200)7		ACTUAL			ALLOWABLE			
MEMBER	SECTION	Ро	Vo	Мо	Ра	Va	Ма	RATIO	
BOTTOM CHORD	1-6CSJ18	0.69C	0.12	4.17	3.69	1.74	6.01	0.89	ASCE 7-16: PASSED
TOP CHORD	1-358CSJ18	0.52T	0.27	2.49	5.69	1.28	4.56	0.61	DESIGN METHOD - (ASD)
WEB	1-358CSJ18	0.32T	0.00	0.21	3.83	0.91	3.90	0.08	COMPONENT WIND PRESSURE DESIGN (END)

BEARING

6.00 2.00

GENERAL NOTES

1. TRUSSES REQUIRE LATERAL BRACING. SEE TRUSS LAYOUT AND DETAIL SHEETS.

2. NUMBER OF FASTENERS NOTED IN CHART INSTALLED ON EACH END OF WEB. 3. ALLOWABLE FASTENER VALUES BASED ON COLD-FORMED STEEL DESIGN FORMULAE.

4. (ws) DENOTES WEB STIFFENER REQUIRED AT SUPPORT.

5. MEMBER DESIGN BASED ON SECTIONS IN UNITED_SSMA-7-8-2016 LIBRARY. 6. SEE ENGINEERS REPORT FOR 'TRUSS TO SUPPORT' CONNECTION DESIGN.

MAXIMUM DEFLECTIONS				
VERTICAL	0.047 in (L/2398)			
HORIZONTAL	0.006 in			
TOP OVERHANG	0.029 in (L/400)			
VERTICAL	0.027 in (L/4587) [D	EAD LOAD ONLY		
VERTICAL	0.022 in (L/5024) [L	IVE LOAD ONLY]		
SUPPORT REACTIONS	DOWN	UPLIFT	HORIZONTAL	B
LEFT	0.47 {0.42}	-0.49 [-0.87]	0.43	6
RIGHT	0.38 (0.38	-0.52 [-1.01]	0.00	2
*UPLIFT LOAD COMBINATIO	N (TRUSS TO SUPPORT	CONNECTION ONLY):	0.6 DEAD + 0.6 WIND	
{ } DENOTES 'DEAD+L	IVE ONLY'			
[] DENOTES 'WIND ON	ILY' UPLIFT REACTION			

DESIGN DATA

PLATE STYLE	: OUT-OF-PLANE
EAVE HEIGHT	: 12.00 ft (TOP OF WALL)
BEARING	: 6.00in
SPACING	: 2.00 ft
DEAD LOAD	: 10.00 PSF (TOP CHORD)
DEAD LOAD	: 10.00 PSF (BOTTOM CHORD)
LIVE LOAD	: 20.00 PSF (TOP CHORD)
LIVE LOAD	: 0.00 PSF (BOTTOM CHORD)
SNOW LOAD	: 0.00 PSF (GROUND) [Ct=1.00]
WIND LOAD	: 35.69 PSF (DESIGN)
WIND SPEED	: 139 MPH (EXPOSURE C)
OPEN CATEGORY	:E
TOPOGRAPHY (Kz)	:1
BUILDING CATEGOR	Y : (2) GENERAL
DELETE WIND LOAD	: RIGHT END
SEISMIC COEFFICIE	NT: 0.0

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2023.03.30

PROJECT: SHOP DWGS FOR EXTERIOR ARCADE

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

 DATE: PROJECT NO.:
 03/30/2023 23-0004

 REVISION
 DATE

 GENERAL REV.
 5/3/2023

NOTES:

TRUSS 14 & 23

SCALE: AS NOTED

SD-8.0

DRAWN BY: CHECKED BY:

TRUSS CONNECTION KEY

SCALE: 1-1/2" = 1'-0"

9

SCALE: 6" = 1'-0"

JM

PJ

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

GENERAL REV. 5/3/2023

DATE:

PROJECT NO .:

REVISION

NOTES:

2023.03.30

03/30/2023

23-0004

DATE

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CONNECTION	SECTION	LENGTH	AXIAL	INT.	FASTENER	PA	REQ.
#1	6CSJ18	6.44	0.62C	0.82	#10 SD	0.000	0
#2	358CSJ18	7.83	0.47C	0.37	#10 SD	0.000	0
#3	358CSJ18	2.58	0.33T	0.05	#10 SD	0.347	8
#4	358CSJ18	4.75	0.57T	0.34	#10 SD	0.177	8
#5	6CSJ18	0.67	0.10C	0.03	#10 SD	0.177	8

CONNECTION	SIMPSON	EACH	LOAD	UPLIFT/SHR	FASTENER	PA	REQ.
CHORD WALL	SEE NOTES			0.33	#10 SD	0.263	8
TRUSS CHORD	SEE NOTES	1	0.33		#10 SD	0.263	8
STEEL STUD	SEE NOTES			0.33	#10 SD	0.263	8
TRUSS CHORD	SEE NOTES	1	0.39		#10 SD	0.263	8
HIP RAFTER	SEE NOTES			0.39	#10 SD	0.263	8

PER AISI S100-2007		ACTUAL			ALLOWABLE				
MEMBER	SECTION	Ро	Vo	Мо	Ра	Va	Ма	RATIO	
BOTTOM CHORD TOP CHORD WEB	1-6CSJ18 1-358CSJ18 1-358CSJ18	0.62C 0.47T 0.57T	0.11 0.13 0.00	2.78 1.57 0.13	2.68 5.91 4.63	1.38 1.38 2.83	4.81 4.71 3.72	0.89 0.46 0.04	ASCE 7-16: PASSED DESIGN METHOD - (ASD) COMPONENT WIND PRESSURE DESIGN (END)

BEARING 6.00 2.00

GENERAL NOTES

1. TRUSSES REQUIRE LATERAL BRACING. SEE TRUSS LAYOUT AND DETAIL SHEETS.

2. NUMBER OF FASTENERS NOTED IN CHART INSTALLED ON EACH END OF WEB.

ALLOWABLE FASTENER VALUES BASED ON COLD-FORMED STEEL DESIGN FORMULAE.
 (ws) DENOTES WEB STIFFENER REQUIRED AT SUPPORT.

5. MEMBER DESIGN BASED ON SECTIONS IN UNITED_SSMA-7-8-2016 LIBRARY. 6. SEE ENGINEERS REPORT FOR 'TRUSS TO SUPPORT' CONNECTION DESIGN.

VERTICAL	0.043 in (L/209	0)		
HORIZONTAL	0.005 in			
TOP OVERHANG	0.026 in (L/500)		
VERTICAL	0.022 in (L/354	4) [DEAD LOAD ONLY]		
VERTICAL	0.017 in (L/479	5) [LIVE LOAD ONLY]		
SUPPORT REACTIONS	DOWN	UPLIFT	HORIZONTAL	В
LEFT	0.36 {0.33}	-0.33 [-0.73]	0.32	6
RIGHT	0.26 (0.26)	-0.37 [-0.79]	0.00	2
*UPLIFT LOAD COMBINAT	ION (TRUSS TO SUPI	PORT CONNECTION ON	LY): 0.6 DEAD + 0.6 W	IND
{ } DENOTES 'DEAD	+LIVE ONLY'			
[] DENOTES 'WIND	ONLY' UPLIFT REACT	ΓΙΟΝ		

DESIGN DATA

NUMBER OF TRUSS =	2 EACH
PLATE STYLE	: OUT-OF-PLANE
EAVE HEIGHT	: 12.00 ft (TOP OF WALL)
BEARING	: 6.000 in
SPACING	: 2.00 ft
DEAD LOAD	: 10.00 PSF (TOP CHORD)
DEAD LOAD	: 10.00 PSF (BOTTOM CHORD)
LIVE LOAD	: 20.00 PSF (TOP CHORD)
LIVE LOAD	: 0.00 PSF (BOTTOM CHORD)
SNOW LOAD	: 0.00 PSF (GROUND) [Ct=1.00]
WIND LOAD	: 35.69 PSF (DESIGN)
WIND SPEED	: 139 MPH (EXPOSURE C)
OPEN CATEGORY	:E
TOPOGRAPHY (Kz)	:1
BUILDING CATEGORY	: (2) GENERAL
DELETE WIND LOAD	: RIGHT END
SEISMIC COEFFICIEN	IT: 0.0

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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

DATE: 03/30/2023 PROJECT NO .: 23-0004 DATE REVISION GENERAL REV. 5/3/2023

NOTES:

TRUSS 15 & 22

SCALE: AS NOTED

SD-9.0

DRAWN BY: CHECKED BY:

3

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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

03/30/2023 DATE: 23-0004 PROJECT NO .: DATE REVISION GENERAL REV. 5/3/2023

NOTES:

TRUSS 15 & 22 CONNECTIONS SCALE: AS NOTED

SD-9.1

DRAWN BY: CHECKED BY:

CONNECTION	SECTION	LENGTH	AXIAL	INT.	FASTENER	PA	REQ.
#1	358CSJ18	4.44	0.39C	0.50	#10 SD	0.000	0
#2	358CSJ18	5.25	0.08C	0.47	#10 SD	0.000	0
#3	358CSJ18	1.75	0.18T	0.09	#10 SD	0.347	8
#4	358CSJ18	3.25	0.39T	0.23	#10 SD	0.177	8
#5	6CSJ18	0.50	0.31C	0.20	#10 SD	0.177	8

CONNECTION	SIMPSON	EACH	LOAD	UPLIFT/SHR	FASTENER	PA	REQ.
CHORD WALL	SEE NOTES			0.24	#10 SD	0.263	8
TRUSS CHORD	SEE NOTES	1	0.24		#10 SD	0.263	8
STEEL STUD	SEE NOTES			0.26	#10 SD	0.263	8
TRUSS CHORD	SEE NOTES	1	0.28		#10 SD	0.263	8
HIP RAFTER	SEE NOTES			0.26	#10 SD	0.263	8

PER AISI S100-200)7		ACTUAL			ALLOWABLE			
MEMBER	SECTION	Ро	Vo	Мо	Ра	Va	Ма	RATIO	
BOTTOM CHORD TOP CHORD	1-358CSJ18 1-358CSJ18	0.39C 0.08T	0.09 0.00	1.89 2.28	3.78 4.98	1.25 1.25	4.83 4.79	0.50 0.49	ASCE 7-16: PASSED DESIGN METHOD - (ASD)
WEB	1-358CSJ18	0.31T	0.00	0.93	4.45	0.91	27.75	0.08	COMPONENT WIND PRESSURE DESIGN (END)

BEARING 6.00 2.00

GENERAL NOTES

1. TRUSSES REQUIRE LATERAL BRACING. SEE TRUSS LAYOUT AND DETAIL SHEETS.

2. NUMBER OF FASTENERS NOTED IN CHART INSTALLED ON EACH END OF WEB.

ALLOWABLE FASTENER VALUES BASED ON COLD-FORMED STEEL DESIGN FORMULAE.
 (ws) DENOTES WEB STIFFENER REQUIRED AT SUPPORT.

5. MEMBER DESIGN BASED ON SECTIONS IN UNITED_SSMA-7-8-2016 LIBRARY. 6. SEE ENGINEERS REPORT FOR 'TRUSS TO SUPPORT' CONNECTION DESIGN.

MAXIMUM DEFLECTIONS						
VERTICAL	0.020 in (L/287	5)				
HORIZONTAL	0.002 in					
TOP OVERHANG	0.023 in (L/500)					
VERTICAL	0.012 in (L/479	2) [DEAD LOAD ONLY]				
VERTICAL	0.009 in (L/8214) [LIVE LOAD ONLY]					
SUPPORT REACTIONS	DOWN	UPLIFT	HORIZONTAL	В		
LEFT	0.26 {0.23}	-0.24[-0.56]	0.24	6		
RIGHT	0.18 (0.18)	-0.26[-0.55]	0.00	2		
*UPLIFT LOAD COMBINATION	ON (TRUSS TO SUPP	PORT CONNECTION ON	LY): 0.6 DEAD + 0.6 W	IND		
{ } DENOTES 'DEAD+	LIVE ONLY'					
[] DENOTES 'WIND O	NLY' UPLIFT REACT	TION				

DESIGN DATA

	S = 2 FACH
PLATE STYLE	: OUT-OF-PLANE
EAVE HEIGHT	: 12.00 ft (TOP OF WALL)
BEARING	: 6.000 in
SPACING	: 2.00 ft
DEAD LOAD	: 10.00 PSF (TOP CHORD)
DEAD LOAD	: 10.00 PSF (BOTTOM CHORD)
LIVE LOAD	: 20.00 PSF (TOP CHORD)
LIVE LOAD	: 0.00 PSF (BOTTOM CHORD)
SNOW LOAD	: 0.00 PSF (GROUND) [Ct=1.00]
WIND LOAD	: 35.69 PSF (DESIGN)
WIND SPEED	: 139 MPH (EXPOSURE C)
OPEN CATEGORY	:E
TOPOGRAPHY (Kz)	:1
BUILDING CATEGO	RY : (2) GENERAL
DELETE WIND LOAD	D : RIGHT END
SEISMIC COEFFICI	ENT: 0.0

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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

DATE: 03/30/2023 PROJECT NO .: 23-0004 DATE REVISION GENERAL REV. 5/3/2023

NOTES:

TRUSS 16 & 21

AS NOTED SCALE:

SD-10.0

DRAWN BY: CHECKED BY:

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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

DATE: PROJECT NO .: REVISION GENERAL REV. 5/3/2023

03/30/2023 23-0004 DATE

NOTES:

TRUSS 16 & 21 CONNECTIONS AS NOTED SCALE:

SD-10.1 DRAWN BY: JM CHECKED BY: PJ

CONNECTION	SECTION	LENGTH	AXIAL	INT.	FASTENER	PA	REQ.
#1	358CSJ18	2.67	0.11C	0.28	#10 SD	0.000	0
#2	358CSJ18	3.17	0.00C	0.11	#10 SD	0.000	0
#3	358CSJ18	1.09	0.17T	0.04	#10 SD	0.347	8
#4	358CSJ18	1.42	0.17T	0.12	#10 SD	0.177	8
#5	6CSJ18	0.50	0.18C	0.13	#10 SD	0.177	8

CONNECTION	SIMPSON	EACH	LOAD	UPLIFT/SHR	FASTENER	PA	REQ.
CHORD WALL	SEE NOTES			0.17	#10 SD	0.263	8
TRUSS CHORD	SEE NOTES	1	0.18		#10 SD	0.263	8
STEEL STUD	SEE NOTES			0.17	#10 SD	0.263	8
TRUSS CHORD	SEE NOTES	1	0.16		#10 SD	0.263	8
HIP RAFTER	SEE NOTES			0.16	#10 SD	0.263	8

PER AISI \$100-200	7		ACTUAL			ALLOWABLE			
MEMBER	SECTION	Ро	Vo	Мо	Ра	Va	Ма	RATIO	
BOTTOM CHORD TOP CHORD WEB	1-358CSJ18 1-358CSJ18 1-358CSJ18	0.11T 0.00C 0.18T	0.17 0.09 0.00	1.18 0.33 0.11	5.62 5.03 4.41	1.23 1.23 0.97	4.69 4.69 27.67	0.28 0.11 0.06	ASCE 7-16: PASSED DESIGN METHOD - (ASD) COMPONENT WIND PRESSURE DESIGN (END)

1. TRUSSES REQUIRE LATERAL BRACING. SEE TRUSS LAYOUT AND DETAIL SHEETS.

2. NUMBER OF FASTENERS NOTED IN CHART INSTALLED ON EACH END OF WEB.

ALLOWABLE FASTENER VALUES BASED ON COLD-FORMED STEEL DESIGN FORMULAE.
 (ws) DENOTES WEB STIFFENER REQUIRED AT SUPPORT.

5. MEMBER DESIGN BASED ON SECTIONS IN UNITED_SSMA-7-8-2016 LIBRARY. 6. SEE ENGINEERS REPORT FOR 'TRUSS TO SUPPORT' CONNECTION DESIGN.

MAXIMUM DEFLECTIONS							
VERTICAL	0.008 in (L/478	6)					
HORIZONTAL	0.000 in						
TOP OVERHANG	0.010 in (L/120	0)					
VERTICAL	0.004 in (L/999	0.004 in (L/9999) [DEAD LOAD ONLY]					
VERTICAL	0.003 in (L/9999) [LIVE LOAD ONLY]						
SUPPORT REACTIONS	DOWN	UPLIFT	HORIZONTAL				
LEFT	0.17 {0.13}	-0.18 [-0.39]	0.16				
RIGHT	0.11{0.11}	-0.16 [-0.32]	0.00				
*UPLIFT LOAD COMBINATIO	ON (TRUSS TO SUPI	PORT CONNECTION ON	LY): 0.6 DEAD + 0.6 V				
{ } DENOTES 'DEAD+	LIVE ONLY						

6.00 2.00 DEAD + 0.6 WIND

BEARING

{ } DENOTES 'DEAD+LIVE ONLY'
 [] DENOTES 'WIND ONLY' UPLIFT REACTION

DESIGN DATA

SEISMIC COEFFICIENT: 0.0

NUMBER OF TRUSS =	2 EACH
PLATE STYLE	: OUT-OF-PLANE
EAVE HEIGHT	: 12.00 ft (TOP OF WALL)
BEARING	: 6.000in
SPACING	: 2.00 ft
DEAD LOAD	: 10.00 PSF (TOP CHORD)
DEAD LOAD	: 10.00 PSF (BOTTOM CHORD)
LIVE LOAD	: 20.00 PSF (TOP CHORD)
LIVE LOAD	: 0.00 PSF (BOTTOM CHORD)
SNOW LOAD	: 0.00 PSF (GROUND) [Ct=1.00]
WIND LOAD	: 35.69 PSF (DESIGN)
WIND SPEED	: 139 MPH (EXPOSURE C)
OPEN CATEGORY	:E
TOPOGRAPHY (Kz)	:1
BUILDING CATEGORY	: (2) GENERAL
DELETE WIND LOAD	: RIGHT END

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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

DATE: 03/30/2023 PROJECT NO.: 23-0004 DATE REVISION GENERAL REV. 5/3/2023

NOTES:

TRUSS 17 & 20

AS NOTED SCALE:

SD-11.0

DRAWN BY: CHECKED BY:

1

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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

03/30/2023 DATE: PROJECT NO .: REVISION GENERAL REV. 5/3/2023

23-0004 DATE

NOTES:

TRUSS 17 & 20 CONNECTIONS AS NOTED SCALE:

SD-11.1 DRAWN BY: JM CHECKED BY: PJ

CONNECTION	SECTION	LENGTH	AXIAL	INT.	FASTENER	PA	REQ.
#1	358CSJ18	0.67	0.02C	0.05	#10 SD	0.000	0
#2	358CSJ18	1.07	0.00C	0.08	#10 SD	0.000	0
#3	358CSJ18	0.58	0.10T	0.08	#10 SD	0.117	8
#4	6CSJ18	0.41	0.01T	0.00	#10 SD	0.177	8

CONNECTION	SIMPSON	EACH	LOAD	UPLIFT/SHR	FASTENER	PA	REQ.
TRUSS CHORD	SEE NOTES	1	0.10		#10 SD	0.263	8
STEEL STUD	SEE NOTES			0.10	#10 SD	0.263	8
TRUSS CHORD	SEE NOTES	1	0.01		#10 SD	0.263	8
STEEL STUD	SEE NOTES			0.01	#10 SD	0.263	8

PER AISI S100-200)7		ACTUAL			ALLOWABLE			
MEMBER	SECTION	Ро	Vo	Мо	Ра	Va	Ма	RATIO	
BOTTOM CHORD	1-358CSJ18	0.02	0.06	0.03	5.03	1.23	4.75	0.05	ASCE 7-16: PASSED
TOP CHORD	1-358CSJ18	0.00T	0.08	0.23	5.71	1.23	4.74	0.08	DESIGN METHOD - (ASD)
WEB	1-358CSJ18	0.01T	0.00	0.17	5.18	1.02	5.78	0.08	COMPONENT WIND PRESSURE DESIGN (END)

1. TRUSSES REQUIRE LATERAL BRACING. SEE TRUSS LAYOUT AND DETAIL SHEETS.

2. NUMBER OF FASTENERS NOTED IN CHART INSTALLED ON EACH END OF WEB.

ALLOWABLE FASTENER VALUES BASED ON COLD-FORMED STEEL DESIGN FORMULAE.
 (ws) DENOTES WEB STIFFENER REQUIRED AT SUPPORT.

5. MEMBER DESIGN BASED ON SECTIONS IN UNITED_SSMA-7-8-2016 LIBRARY. 6. SEE ENGINEERS REPORT FOR 'TRUSS TO SUPPORT' CONNECTION DESIGN.

MAXIMUM DEFLECTIONS VERTICAL HORIZONTAL	0.000 in (L/9999) 0.000n			
SUPPORT REACTIONS	DOWN	UPLIFT	HORIZONTAL	BEARING
LEFT	0.07 {0.06}	-0.10 [-0.19]	0.06	3.00
RIGHT	0.01 {0.01}	-0.01 [-0.03]	0.00	3.00
*UPLIFT LOAD COMBINATIO	N (TRUSS TO SUPPOR	T CONNECTION ONLY):	0.6 DEAD + 0.6 WIND	

{ } DENOTES 'DEAD+LIVE ONLY'[] DENOTES 'WIND ONLY' UPLIFT REACTION

DESIGN DATA

NUMBER OF TRUSS =	2 EACH
PLATE STYLE	: OUT-OF-PLANE
EAVE HEIGHT	: 12.00 ft (TOP OF WALL)
BEARING	: 6.000 in
SPACING	: 2.00 ft
DEAD LOAD	: 10.00 PSF (TOP CHORD)
DEAD LOAD	: 10.00 PSF (BOTTOM CHORD)
LIVE LOAD	: 20.00 PSF (TOP CHORD)
LIVE LOAD	: 0.00 PSF (BOTTOM CHORD)
SNOW LOAD	: 0.00 PSF (GROUND) [Ct=1.00]
WIND LOAD	: 35.69 PSF (DESIGN)
WIND SPEED	: 139 MPH (EXPOSURE C)
OPEN CATEGORY	:E
TOPOGRAPHY (Kz)	:1
BUILDING CATEGORY	: (2) GENERAL
DELETE WIND LOAD	: RIGHT END
SEISMIC COEFFICIEN	T: 0.0

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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

DATE: 03/30/2023 PROJECT NO .: 23-0004 DATE REVISION GENERAL REV. 5/3/2023

NOTES:

-----_____

TRUSS 18 & 19

AS NOTED SCALE:

SD-12.0

DRAWN BY: CHECKED BY: JM

PJ

TRUSS CONNECTION KEY

SCALE: 1-1/2" = 1'-0"

SCALE: 6" = 1'-0"

(4) HEX HEAD GRABBERS #10x3/4"

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2023.03.30

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

03/30/2023 DATE: PROJECT NO .: REVISION GENERAL REV. 5/3/2023

23-0004 DATE

NOTES:

_____ _____

TRUSS 18 & 19 CONNECTIONS AS NOTED SCALE:

SD-12.1 JM DRAWN BY:

PJ

CHECKED BY:

#12 TEK SCREWS (MIN.) @ 6" O.C. —

- WOOD PANEL 4'-0"x8'-0" STAGGERED JOINT PATTERN

0

STATE OF FLORIDA PARTIAL ROOF **TRUSS PLAN** AS NOTED SCALE: SD-13.0

5/17/2023 PROFESSIONAL ENGINEER SEAL

NOTES:

WINTER GARDEN, FL 34787 03/30/2023 DATE: PROJECT NO .: 23-0004 DATE REVISION GENERAL REV. 5/3/2023

130 RUBY RED PLACE FC,

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

2023.03.30

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DRAWN BY:

CHECKED BY:

TYP. ROOF ASSEMBLY CONNECTION DETAILS

1500 SERIES PERFORMANCE/SUBMITTAL

APPROVALS/LISTINGS

City of Los Angeles

#RR-22668 Powder pins

ICC Evaluation Service, Inc.

#ESR-1799 Powder Pins & Clips

Ramset fasteners may be specified by their type or catalog number to satisfy fastening requirements.

PIN SPECIFICATIONS

- Made from AISI 1070 steel. Austempered to a core hardness of 52-56 Rc
- Typical tensile strength: 270,000 psi Typical shear strength: 162,000 psi
- STANDARD FINISHES
- Mechanical zinc plate to a minimum thickness of .0002 meets requirements

of ASTM B695—Class 5 Type 1

FASTENERS IN NORMAL WEIGHT CONCRETE											
PART NUMBER SERIES	SHANK	MINIMUM PENETRATION (INCH)		INSTALLED IN STONE AGGREGATE CONCRETE CONCRETE COMPRESSIVE STRENGTH ALLOWABLE LOAD - Ultimate Load							
	(INCH)		2000) PSI	4000	PSI	6000 PSI				
SEMES			TENSION (LBS)	SHEAR (LBS)	TENSION (LBS)	SHEAR (LBS)	TENSION (LBS)	SHEAR (LBS)			
		3/4	50 655	66 739	100 <i>511</i>	104 552					
1500/1600 SERIES	0.145	1	152 943	166 <i>1229</i>	157 <i>9</i> 37	182 1342					
	0.145	1-1/4	159 1078	265 1665	179 1043	267 1538					
		1-1/2	154 1450	340 2027	209 1357	342 1712					

FASTENERS IN LIGHTWEIGHT CONCRETE

PART	SHANK MINIMUM		ALLOWABLE WORKING VALUES INSTALLED IN 3000 PSI LIGHTWEIGHT CONCRETE ALLOWABLE LOAD - Ultimate Load							
SERIES	(INCH)	(INCH)	3000 PSI LIGHT	VEIGHT W/DECKING	3000 PSI LIGHTWEIGHT					
			LOWER FLUTE TENSIO	N LOWER FLUTE SHEAR	TENSION	SHEAR				
	0.145	3/4	76 395	260 1409	167 837	179 894				
		1	134 668	265 1505	200 998	228 1141				
ISUU SERIES		1-1/4	157 784	269 1344	333 1664	400 2090				
		1-1/2	233 1163	346 1728	391 1957	410 2050				
Note 1: ALLOWABLE	ote 1: ALLOWABLE loads are shown in the LARGE BOLD font, Ultimate loads are shown in smaller italic font. Note 2: Testing conducted in accordance with ICC AC70 & ASTM E1190. Note 3:									

members must be investigated separately. Note 5: Cyclic, fatigue, shock loads, and other design criteria may require a different safety factor. Note 6: Job site testing may be required to determine actual job site values. Note 7: Minimum edge distance is 3 inches unless otherwise approved. Note 8: For SI: 1 lbf = 4.448 N, 1 inch = 25.4 mm, 1 ksi = 6.89MPa

FASTE	FASTENERS IN STEEL											
PART	SHANK			INSTALLED IN A36 STRUCTURAL STEEL-STEEL THICKNESS (INCHES) ALLOWABLE LOAD - Ultimate Load								
NUMBER	DIA		3/1	16	1,	/4	3/	8	1/	2	3/4	k i i
SERIES	(INCH)	SHANK	TENSION (LBS)	SHEAR (LBS)	TENSION (LBS)	SHEAR (LBS)	TENSION (LBS)	SHEAR (LBS)	TENSION (LBS)	SHEAR (LBS)	TENSION (LBS)	SHEAR (LBS)
1500/	0.145	SMOOTH	81 790	373 2039	181 1269	273 1642	397 2169	489 2771	243 1328 ⁸	277 1514 ⁸		
1600	0.145	KNURLED	296 1633	636 3516	584 3384	659 3822	680 3755	730 4030	253 1459 ⁸	293 1632 ⁸		
PART	SHANK	TYPE OF		INSTAL	LED IN A57	72 GRADE ALLO	50 STRUCT WABLE LOA	URAL STEE	L-STEEL TH te Load	ICKNESS (II	NCHES)	
PART NUMBER	SHANK DIA	TYPE OF	3/1	INSTAL 6	LED IN A57	72 GRADE ALLO	50 STRUCT WABLE LOA 3	URAL STEE D - Ultima 8	L-STEEL TH te Load 1	ICKNESS (II) /2	NCHES)	ŀ
PART NUMBER SERIES	SHANK DIA (INCH)	TYPE OF SHANK	3/1 TENSION (LBS)	INSTAL 6 SHEAR (LBS)	LED IN A57 1/ TENSION (LBS)	72 GRADE ALLO 4 SHEAR (LBS)	50 STRUCT WABLE LOA 3, TENSION (LBS)	URAL STEE .D - Ultimat /8 SHEAR (LBS)	L-STEEL TH te Load 1 TENSION (LBS)	ICKNESS (IN /2 SHEAR (LBS)	NCHES) 3/4 TENSION (LBS)	SHEAR (LBS)
PART NUMBER SERIES	SHANK DIA (INCH)	TYPE OF SHANK	3/1 TENSION (LBS)	INSTAL 6 SHEAR (LBS)	LED IN A57 1/ TENSION (LBS)	72 GRADE ALLO 4 SHEAR (LBS)	50 STRUCT WABLE LOA 3 TENSION (LBS)	URAL STEE D - Ultimat /8 SHEAR (LBS)	L-STEEL TH te Load 1 TENSION (LBS)	/2 SHEAR (LBS)	3/4 TENSION (LBS)	SHEAR (LBS)
PART NUMBER SERIES	SHANK DIA (INCH) 0.145	TYPE OF SHANK SMOOTH KNURLED	3/1 TENSION (LBS) 260 1609	INSTAL 6 SHEAR (LBS) 499 3182	LED IN A57 1/ TENSION (LBS) 579 3411	72 GRADE ALLO 4 5HEAR (LBS) 725 4272	50 STRUCT WABLE LOA 3, TENSION (LBS) 383 2216 ⁷	URAL STEE D - Ultimat /8 SHEAR (LBS) 595 3431 ⁷	L-STEEL TH te Load 1 TENSION (LBS) 	/2 SHEAR (LBS) 	3/2 TENSION (LBS)	SHEAR (LBS)

Safety factors are based on coefficient of variation. In accordance with ICC AC70, the safety factor will be no less than 5. Note 4: Cyclic, fatigue, shock loads, and other design criteria may require a different safety factor. Note 5: Job site testing may be required to determine actual job site values. Note 6: Values shown are for fastenings that have the entire pointed end of the fastener driven through the steel plate; except as noted below. Note 7: Fastener penetration is 3/8" minimum. Note 8: Fastener penetration is 7/16" minimum. Note 9: Fastener penetration is 1/2" minimum Note 10: For SI: 1 lbf = 4.448 N, 1 inch = 25.4 mm, 1 ksi = 6.89MPa. * Partial penetration = .28

yield strength, 65ksi (450MPa) minimum tensile strength, 68mil minimum thickness (14 gauge, 0.0713" design thickness) with ASTM A653/A653M G90 (Z275) hot dipped galvanized coating. The attachment of StiffClip to the primary structure may be made with PAFs, screw/ bolt anchors or weld and is dependent upon the base material (steel or concrete) and the design configuration.

StiffClip AL Allowable Loads AL800 StiffClip[®] AL, Recommended Allowable Load (lbs): F1 & F2 F1 Load Direction Stud AL362 AL600 AL800 hickness Yield w/3 #12 w/3 #12 w/4 #12 w/2 #12 w/3 #12 w/4 #12 w/2 #12 w/3 #12 w/4 #12 w/2 #12 w/4 #12 w/4 #12 w/6 #12 Mils (ga) Strength (ksi) icrews Screws <u>191</u> <u>191</u> <u>191</u> <u>377</u> <u>490</u> <u>754</u> <u>377</u> <u>463</u> <u>752</u> <u>377</u> <u>754</u> <u>1,131</u> 33 (20) 33 33 (20) 275 275 275 544 708 1,089 544 670 1,089 544 1,089 1,633 50 43 (18) 248 248 248 561 729 1,122 560 690 1,120 561 1,122 1,683 33 43 (18) 50 359 359 359 810 1,053 1,470 810 997 1,620 810 1,620 2,430 312 312 312 789 1,025 1,470 788 970 1,577 789 1,577 2,366 54 (16) 33 54 (16) 50 450 450 450 1,139 1,470 1,470 1,138 1,401 2,091 1,139 2,278 2,516 50 567 567 567 1,470 1,470 1,470 1,610 1,981 2,091 1,610 2,516 2,516 68 (14) 50 809 809 809 1,470 1,470 1,470 1,698 2,089 2,091 1,698 2,516 2,516 97 (12)

Max Allowable Clip Load 975 866 1,768

	StiftClip AL, Recommended Allowable Load (Ibs): F5									
Stu	d	F3 Load Direction								
Viold		AL362				AL600		AL800		
Thickness Mils (ga)	Strength (ksi)	w/2 #12 Screws	w/3 #12 Screws	w/4 #12 Screws	w/2 #12 Screws	w/3 #12 Screws	w/4 #12 Screws	w/2 #12 Screws	w/4 #12 Screws	w/6 #12 Screws
33 (20)	33	256	409	511	324	495	650	347	692	987
33 (20)	50	370	591	738	468	716	939	501	999	1,426
43 (18)	33	381	609	760	482	737	967	516	1,029	1,469
43 (18)	50	551	879	1,098	697	1,065	1,398	745	1,487	2,123
54 (16)	33	536	856	1,069	678	1,037	1,360	726	1,447	2,066
54 (16)	50	775	1,236	1,543	980	1,498	1,965	1,048	2,090	2,984
68 (14)	50	1,095	1,747	2,182	1,385	2,118	2,778	1,482	2,955	4,219
97 (12)	50	1,155	1,842	2,301	1,460	2,233	2,929	1,562	3,116	4,449
118 (10)	50	1,155	1,842	2,301	1,460	2,233	2,929	1,562	3,116	4,449
Maximum Allowable Clip Load			2,458			3,015			6,128	

**Important notes for StiffClip AL Allowable Load tables continued on next page.

"The Professional's Choice"

NII

Heavy Gauge Metal (20ga or thicker)

Product #246 Hex Head DRIVALL® Self Drilling (1 OF 3)

Attaches fixtures, backup plates, door frames and lathers channel to structural studs, meta checks, etc. 20-8 ga.

H

Hex Head

S-12 Drill Point

#3 Drill Point

Finish - Clear Zinc and GRABBERGARD

- Steel studs to track, 20-8 gauge steel
- Steel studs to door frame jamb anchor clips
- Plaster grounds to 20-8 gauge steel
- Other metal-to-metal attachments, 20-8 gauge

SPECIFICATIONS

- Gauge #6 to #14
- Lengths 1/2" to 2-1/2"
- Head Type Hex
- Thread Type Single Lead
- Finish Clear Zinc
- GRABBER[®] screws are manufactured in an ISO 9001 and ISO 14001 certified and approved factory. Specific fasteners are listed in ICC ESR-1271; please check the report for listed item numbers.

PRODUCT FEATURES

#4 Drill Point

- > Self tapping drill point is designed for penetration into heavy gauge metal • Hex head provides maximum driving torque with a flat bearing surface
- Also available with #3 point

 \bigcirc

Hex Drive

INSTALLATION GUIDELINES

- Use a standard screwgun with a depth-sensitive nose piece. Suggested screwgun specification for optimal performance – 4 amps minimum and RPM range of 0 to 2,500.
- > The Hex head is fully seated when the head is flush with the work surface.
- Overdriving may result in failure of the fastener or stripout of the work surface.
- > The fastener must penetrate beyond the metal a minimum of three thread pitches.

GRABBER CONSTRUCTION PRODUCTS

Page 75 | StiffClip[®] AL

eavy Gauge Metal (20ga or thicker)

Product #246

Hex Head DRIVALL® Self Drilling (2 OF 3)

				• • •
PRODU	CT SIZES AN	D ORDERIN	G INFORMATIO	N
Catalog No.	Gauge/Length	Length Metric	Quantity Per Carton	Weight Per Carton
08100H3	#8x1"	25 mm	7.5 M	38.00(lbs)
08100HA	#8x1″	25 mm	7.5 M	38.00(lbs)
08125H3	#8x1-1/4"	32 mm	5 M	35.00(lbs)
08050H3	#8x1/2"	13 mm	15 M	37.00(lbs)
08200H3	#8x2"	51 mm	3 M	33.00(lbs)
08075H3	#8x3/4"	19 mm	10 M	42.00(lbs)
08058H3	#8x5/8"	16 mm	10 M	37.00(lbs)
10100H3	#10x1"	25 mm	5 M	37.00(lbs)
10150H3	#10x1-1/2"	38 mm	3.5 M	35.00(lbs)
10125H3	#10x1-1/4"	32 mm	5 M	33.00(lbs)
10050H3	#10x1/2"	13 mm	7.5 M	37.00(lbs)
10200H3	#10x2"	51 mm	3 M	31.00(lbs)
10300H3	#10x3"	76 mm	3 M	38.00(lbs)
10075H3	#10x3/4"	19 mm	5 M	30.00(lbs)
10058H3	#10x5/8"	16 mm	7.5 M	41.00(lbs)
12100H3	#12x1″	25 mm	3.5 M	40.00(lbs)
12150H3	#12x1-1/2"	38 mm	2.5 M	35.00(lbs)
12125H3	#12x1-1/4"	32 mm	3.5 M	34.00(lbs)

12150H3	#12x1-1/2"	38 mm	2.5 M	35.00(lbs)
12125H3	#12x1-1/4"	32 mm	3.5 M	34.00(lbs)
12250H3	#12x2-1/2"	63 mm	1.5 M	30.00(lbs)
12300H3	#12x3″	76 mm	1 M	22.00(lbs)
12350H3	#12x3-1/2"	89 mm	1 M	21.00(lbs)
12075H3	#12x3/4"	19 mm	5 M	40.00(lbs)
12075HA	#12x3/4"	19 mm	5 M	40.00(lbs)
14100H3	#14x1″	25 mm	3.5 M	52.00(lbs)
14150H3	#14x1-1/2"	38 mm	2 M	33.00(lbs)
14125H3	#14x1-1/4"	32 mm	2 M	35.00(lbs)
14200H3	#14x2"	51 mm	1.5 M	34.00(lbs)
14250H3	#14x2-1/2"	63 mm	2 M	33.00(lbs)
14300H3	#14x3"	76 mm	2 M	22.00(lbs)
14075H3	#14x3/4"	19 mm	5 M	61.00(lbs)
14400H4	#14x4″	102 mm	.5 M	21.00(lbs)
1224150H4RG	#12-24x1-1/2"	38 mm	2.5 M	33.00(lbs)
1224125H4	#12-24x1-1/4"	32 mm	3 M	50.00(lbs)
1224200H4RG	#12-24x2-1/2"	63 mm	2 M	36.00(lbs)
122478H4	#12-24x7/8"	22.23 mm	3.5 M	50.00(lbs)

101

GRABBER Construction Products 102

The Steel Network, Inc.

StiffClip[®] Al Multi-Directional Load Resistant Angle

ASTM A1003/A1003M Structural Grade 50 (340)

Type H, ST50H (ST340H): 50ksi (340MPa) minimum

Material Composition

The Steel Network, Inc.

StiffClip[®] AL | www.steelnetwork.com/Product/StiffClipAL

1-888-474-4876

AL800

2.516

The Steel Network, Inc.

www.steelnetwork.com 🞞

- Notes - Allowable load tables incorporate eccentric loading of fasteners attached 3/4" from the heel of the clip. Values with welded
- connection may increase. - The attachment of the 1.5" leg of StiffClip AL is dependent on the allowable loads of the fasteners, and is to be designed by others. - Fasten within $\frac{34''}{4}$ from the angle heel (centerline of the $\frac{112''}{4}$ leg).
- All guide holes may not require fasteners. Number of fasteners used is to be determined by designer.
- Stiffening ribs are not present in the AL800. - StiffClip AL is tested to resist loads in horizontal, vertical, and lateral directions.
- Loads listed reflect force in a single direction. When multiple loads react on the connection, it is the responsibility of the designer to check the interaction of forces. Allowable loads are for attachment through 3" leg only. Attachment through 1.5" leg should be engineered. (See material
- composition above for calculation purposes.)
- Allowable loads have not been increased for wind, seismic, or other factors. - Torsional effects are considered on screw group for F2 & F3 allowable loads. It is assumed that half of the torsional moment is taken by the connection to the structure and half is taken by the connection to the stud.

Screw Patterns AL362/600	AL80	00					đ	
	•	0	٠		0	٠	•	o 🌑 9i
	•	o	o		0	•	•	n n n
2 Screw Pattern 3 Screw Pattern 4 Screw Pattern		2 Screw Pattern		4 9	Screw Patte	rn	6	Screw Pattern

Nomenclature StiffClip AL is available for various stud depths. To specify, multiply stud depth by 100.*

Example: 6" stud depth

Designate: StiffClip® AL600

* The AL362 fits 3 ⁵/₈" and 4" member depths ** Stiffening ribs are not present in the AL800.

Example Details

** For more information or to review a copy of this report, please visit our website at http://www.steelnetwork.com/Site/TechnicalData

www.steelnetwork.com | 1-888-474-4876

AL362 & AL600

F2 Load Direction

AL600

2.091

Load Direction

3.125

AL362

118 (10) 50 856 856 856 1,470 1,470 1,470 1,698 2,089 2,091 1,698 2,516 2,516

1,470

e Load (lbs): F3

062017 | The Steel Network, Inc.

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StiffClip[®] AL | Page 76

"The Professional's Choice"

GRABBER CONSTRUCTION PRODUCTS

leavy Gauge Metal (20ga or thicker) Product #246

Hex Head DRIVALL® Self Drilling (3 OF 3)

SHEET S	TEEL GAUGE	S	
Gauge No.	Decimal E	quivalent Millime	eter Equivalent
14	0.0713″	1.8 mn	n
16	0.0566″	1.5 mn	n
18	0.0451″	1.2 mn	n
20	0.0396″	1 mm	
Finish Clear Zinc	Test Salt Spray Results	Standard/Protocol ASTM B117	Results 24 hours, no red rust
GRABBERGAR	Results D® Salt Spray	ASTM B117	no red rust 1,000 hours,
GRABBERGAR	Results D® Kesternich Results	DIN 50018, 2.0L	15 cycles, no red rust

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$ \stackrel{D}{\longleftrightarrow} $	U	

DRILLING CAPACITY							
Screw Gauge	Drill Point	Max Panel Thickness	Steel Thickness Gauge Range				
8	3	0.100-0.140	20-12				
10	3	0.110-0.175	20-8				
12	3	0.110-0.210	20-8				
14	3	0.110-0.220	20-8				
12	4	0.175-0.220	18-0				
14	4	0.175-0.250	18-1/4"				
12	5	0.250-0.500	18-1/2"				

PULL-OUT	TEST D	ATA		
Screw Gauge	TPI	Metal Gauge	Tension (lbs.)	Shear (lbs.)
#8	18	14	450	1012
		16	442	955
		18 20	340 265	1045 680
#10	16	14	657	1395
		16	542	1332
		18	415	1072
		20	367	675
#12	14	14	577	1827
		16	582	1377
		18	400	1087
		20	350	670
#14	14	14	602	1995
		16	560	1485
		18	460	1127
		20	240	748
**GRABBFR® fa	steners are r	ot categorized as struct	ural holts. The figures l	isted above

are ultimate average values achieved under independent laboratory conditions, and apply to GRABBER® Line fasteners only. An appropriate safety factor must be determined by a qualified professional for design purposes. \sim

SHEET STEEL GAUGES							
Gauge No.	Decimal Equivalent	Millimeter Equivalent					
12	0.1017″	2.6 mm					
14	0.0713″	1.8 mm					
16	0.0566″	1.5 mm					
18	0.0451″	1.2 mm					
20	0.0396″	1 mm					

PRODUCT DIMENSIONS (MILLIMETERS)										
Gauge	B Head Dia	Recess Depth	W Gauging Width	d Shank Dia	d1 Minor Dia	d2 Major TPI Dia				
#8	8.18	6.20	3.28	2.85	4.05	18				
	8.84	6.35	3.32	3.05	4.25					
#10	9.75	7.75	3.78	3.25	4.62	16				
	10.50	7.93	3.82	3.45	4.82					
#12	10.10	7.75	4.38	3.99	5.31	14				
	11.00	7.93	4.42	4.17	5.46					
#14	12.20	9.32	5.08	4.70	6.10	14				
	13.20	9.52	5.12	4.88	6.25					

All GRABBER® screw products are manufactured in facilities that are ISO 9001 certified. Specific fasteners are listed in ICC ESR-1271; please check the report for listed item numbers. ©2012 GRABBER Construction Products, Inc. GRABBER®, STREAKER®, DRIVALL®, LOX®, GRABBERGARD® and SCAVENGER® are registered trademarks of Grabber Construction Products, Inc.

"The Professional's Choice"

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PHILADELPHIA, PA 19103

2023.03.30 PROJECT:

SHOP DWGS FOR **EXTERIOR ARCADE**

130 RUBY RED PLACE FC. WINTER GARDEN, FL 34787

DATE: 03/30/2023 PROJECT NO .: 23-0004 DATE REVISION GENERAL REV. 5/3/2023

NOTES:

SPECIFICATIONS

AS NOTED SCALE:

SD-15.0 DRAWN BY:

CHECKED BY:

PJ

3-5/8" STUD/TRACK PROPERTIES

Effective Propertie Member Design thickness (in) Lu (in) strength Fy (ksi) 362S137-33 0.0346 0.804 0.694 34.7 0.236 0.479 0.264 0.0594 0.697 0.700 362S137-43 0.0451 0.306 1.04 0.616 0.340 1.42 0.0755 0.497 0.616 0.320 6.33 0.207 0.608 362S137-54 0.0566 0.379 0.756 0 4 1 7 0.0911 0.490 0.756 0.405 0.251 -0.978 0.601 27.9 1.29 362S137-68 0.0713 0.470 0.509 0.109 0.481 0.704 0.923 -0.959 362S137-97 0.1017 0.648 2.20 1.23 0.678 1.38 0.138 0.461 1.23 0.663 0.390 -0.922 0.713 2.23 27.8 0.268 0.372 0.592 0.594 0.597 0.262 0.340 0.304 0.392 362S162-33 0.0346 0.892 0.551 1.45 0.099 0.127 0.616 0.611 0.551 0.710 5.29 0.105 0.297 0.789 42.6 5.43 362S162-43 0.0451 0.710 0.376 0.782 42.5 1.16 1.45 7.34 0.230 0.422 362S162-54 0.0566 0.873 0.482 1.44 0.154 0.605 0.444 0.451 34.4 1.44 0.873 362S162-68 0.0713 0.524 0.590 0.186 0.596 0.574 0.600 1.43 362S162-97 0.724 0.60 362S200-33 0.0346 0.297 0.177 0.772 0.478 0.648 0.642 362S200-43 0.0451 0.385 0.836 0.461 0.227 0.767 0.836 0.427 8.43 0.480 0.261 0.734 0.482 362S200-54 0.0566 0.479 0.568 0 277 0.761 0.490 362S200-68 0.0713 0.595 0.698 0.337 0.753 0.484 362S200-97 0.1017 0.826 0.945 0.446 0.735 0.929 0.487 2.81 1.44 0.430 0.535 362S250-43 0.0451 0.541 0.385 0.396 1.46 0.980 0.946 0.980 0.449 8.88 362S250-54 0.0566 0.668 0.473 0.940 0.514 0.666 362S250-68 0.0713 0.823 0.578 362S250-97 0.1017 0.927 362T125-33 0.0346 362T125-430.0451362T125-540.0566 0.276 0.838 0.841 0.846 0.0481 0.346 0.378 0.373 0.369 -0.648 0.404 1.18 362T125-68 0.0713 0.436 1.48 0.475 0.0596 0.908 0.738 0.156 -0.641 0.399 362T125-97 0.1017 0.854 0.621 0.675 0.0822 -0.626 1.34 0.364 0.226 0.390 2.11 1.47 1.34 0.675 2.14 1.64 0.766 0.768 362T150-33 0.0346 0.229 0.264 -0.854 0.499 0.0499 0.467 0.180 0.124 0.522 0.780 1.48 0.414 362T150-43 0.0451 0.298 0.650 0.344 0.0644 1.48).465 362T150-54 0.0566 362T150-68 0.0713 0.0801 0.100 0.138 0.374 0.823 0.431 1.48 0.462 -0.84 0.460 0.453 0.449 0.733 0.471 0.542 0.771 1.49 0.993 0.799 0.257 -0.836 0.511 0.777 0.787 362T150-97 0.1017 0.501 1 78 0.672 2 29 1 5 1 21.9 0.374 -0.820 362T200-33 0.0346 0.264 0.897 0.619 0.329 0.110 0.142 0.177 0.645 0.464 0.190 3.76 0.105 0.269 0.754 0.752 0.748 2.09 0.631 0.633 0.638 0.343 0.431 0.643 0.640 0.350 0.442 362T200-43 0.0451 1.17 0.808 0.427 0.650 0.270 0.233 2.09 1.53 5.34 362T200-54 0.0566 0.536 0.345 0.460 1.47 1.54 0.832 10.3 2.09 1.02 -1.26 0.221 0.638 362T200-68 0.0713 0.543 0.675 0.480 0.919 0.564 0.743 0.643 1.55 1.14 14.4 2.09 362T200-97 0.1017 0.773 1.92 0.963 1.57 0.308 0.632 1.84 0.804 24.1 — 2.67 0.825 0.732 2.10 0.655 2.63 -1.23 50 _ 0.544 0.685 0.439 0.443 362T300-54 0.0566 0.746 0.531 0.988 0.985 0.371 1.85 1.43 0.581 2.86 1.62 362T300-68 0.0713 0.941 0.665 0.985 1.36 0.519 2.86 2.33 362T300-97 0.1017 0.977 1.35 0.937 0.979 3.32 2.68 1.66 'Web-height-to-thickness ratio exceeds 200. Web stiffeners are required at all support points and concentrated loads *Allowable moment includes cold work of forming.

Gross Properties: Ix = Moment of Inertia of cross-section about the x-axis. Sx = Section Modulus about the x-axis. Rx = Radius of Gyration of cross-section about the x-axis. Iy = Moment of Inertia of cross-section about the y-axis. Ry = Radius of Gyration of cross-section about the y-axis.	Effective Properties: Ixe = Effective Moment of Inertia of cross-section about the x-axis. Sxe = Effective Section Modulus about the x-axis. Ma = Allowable Moment based on local buckling. Mad = Allowable Moment based on distortional buckling, assuming Kφ=0.	Torsional and Other Properties: J = St. Venant Torsional Constant. The values of J shown in the tables have been factored by 1000. Ro W = Warping Torsion Constant. Lu Xo = Distance from shear center to the centroid along the principal axis. m m = Distance from shear center to mid-plane of web.	 Polar Radius of Gyration of cross-section about the shear center. a 1-(Xo/Ro)² Critical unbraced length for lateral-torsional buckling. Members are considered fully braced when unbraced length is less than Lu. 	 Notes for Allowable Stud Shear: Capacities are calculated per Allowable Studies of the star For perforated sections, the star 				
	Complies with AISI S100-12 • IBC 2015							
Pub. No. CD-STR-TechGuide 6/17	The technical content of this literature is effective	e 6/1/17 and supersedes all previous information.	clarkdietrich.com	Pub. No. CD-STR-TechGuide 6/17				

connectors for Cold-Formed Steel Construction HRS/ST/FHA/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI Strap Ties

Straps are load rated and provide the correct thickness and number of fasteners the specifier is looking for compared with field fabricated straps. Install strap ties where top or bottom plates are cut, at wall intersections, and as ridge ties. Reduce the allowable load based on the size and quantity of fasteners used. Refer to applicable code for minimum edge and end distances.

1. PS strap design loads must be determined by the designer for each illustration. Hole diameter in the part may be oversized to accommodate the HDG. Designer must determine if the oversize creates an unacceptable installation.

Finish: PS - hot-dip galvanized (HDG); all others - galvanized. Some products are available in stainless steel or ZMAX®; see Corrosion Information, pp. 19–23. Installation: Use all specified fasteners; see General Notes Codes: See p. 13 for Code Reference Key Chart

Connectors for Cold-Formed Steel Construction

2. Loads are based on lesser of steel capacity or fastener calculation. 3. Not all fastener holes need to be filled, as additional fastener holes are provided. Install fasteners symmetrically.

31/4" 11/2"

ALLC	WABL	E STUI	DSHEA	R (LBS)			
Mei	nber	Solid	Perforated	Me	Member		Perforated
Size (in)	mil (ksi)	Section	Section	Size (in)	mil (ksi)	Section	Section
	33 (33)	975	399		33 (33)	638	638
25	43 (33)	1265	394		43 (33)	1416	1240
2.5	54 (50)	2353	565	6	54 (50)	2823	1947
	68 (50)	2866	519		68 (50)	5350	2879
	33 (33)	1024	487		97 (50)	10472	3806
25	43 (33)	1739	631		33 (33) ¹	474	_
3.5	54 (50)	3372	947		43 (33)	1051	1051
	68 (50)	4203	897	8	54 (50)	2091	2091
	33 (33)	1024	521		68 (50)	4221	3367
	43 (33)	1739	676		97 (50)	10885	5938
3.625	54 (50)	3372	1016		43 (33) ¹	836	_
	68 (50)	4370	1004	10	54 (50)	1661	1661
	97 (50)	5943	875	10	68 (50)	3345	3345
	33 (33)	976	595		97 (50)	9864	7177
	43 (33)	1739	810		54 (50) ¹	1377	
4	54 (50)	3372	1223	12	68 (50)	2771	2771
	68 (50)	4871	1356		97 (50)	8147	7411
	97 (50)	6658	1207		54 (50) ¹	1177	_
	33 (33)	699	699	14	68 (50)	2365	2365
	43 (33)	1550	1199		97 (50)	6939	6939
5.5	54 (50)	3093	1881				
	68 (50)	5350	2532				
	97 (50)	9518	3026				

¹Web-height to thickness ratio exceeds 200. Web stiffeners are required at all support points and concentrated loads.

ALLOWABLE TRACK SHEAR (LBS)

Member		Solid Section	Me	ember	Solid Section
Size (in)	mil (ksi)	Solid Section	Size (in)	mil (ksi)	Solid Section
	33 (33)	1024		33 (33)	622
2.5	43 (33)	1356		43 (33)	1377
2.5	54 (50)	2563	6	54 (50)	2728
	68 (50)	3199		68 (50)	5350
	33 (33)	1024		97 (50)	10885
2.5	43 (33) 1739 54 (50) 3372		33 (33) ¹	465	
3.5 54 (5	54 (50)	3372	8	43 (33)	1030
	68 (50)	4536		54 (50)	2039
	33 (33)	1024		68 (50)	4087
	43 (33) 1739	1739		97 (50)	10885
3.625	54 (50)	3372		43 (33) ¹	822
	68 (50)	4703	10	54 (50)	1628
	97 (50) 6622	6622	10	68 (50)	3261
	33 (33)	940		97 (50)	9507
	43 (33)	1739		54 (50) ¹	1354
4	54 (50)	3372	12	68 (50)	2713
	68 (50)	5205		97 (50)	7902
	97 (50)	7337		54 (50) ¹	1160
	33 (33)	680	14	68 (50)	2322
	43 (33)	1504		97 (50)	6761
5.5	54 (50)	2980		. ,	
	68 (50)	5350			
	97 (50)	10197			

Product category: Product name:	S162 (1-5/8" Flan 250S162-33 (33k	ge Structural Stu si, CP60) P - Pu	ıd) nched
	33mils (20ga)	Coating: Color coding:	CP60 per AISI S240 White
Geometric Propert	ies	-	
Web depth 2.50	00 in		
Flange width 1.62	25 in Punchou	t width	0.75 in
Stiffening lip 0.50	0 in Punchou	t lenath	4.00 in
Design thickness 0.03	346 in Min. stee	l thickness	0.0329 in
Yield strength. Fv 33 l	si Fv with C	old-Work. Fva	33.0 ksi
Ultimate, Fu 45.0) ksi	···· · · · · · · · · · · · · · · · · ·	
Gross Section Prop	erties of Full Sec	tion, Strong	Axis
Cross sectional area (A)			0.223 in ²
Member weight per foot of	length		0.76 lb/ft
Moment of inertia (Ix)			0.235 in ⁴
Section modulus (SX)			0.188 in ³
Gross moment of inertia (N	ı)		1.027 m^4
Gross radius of gyration (F	() (y)		0.624 in
Effective Section P	roperties, Strong	j Axis	
Effective Area (Ae)			0.172 in ²
Moment of inertia for defle	ction (Ix)		0.235 in ⁴
Section modulus (SX) Allowable bonding momon	t (Ma)		0.180 m ³
Allowable benuing moment	n distortion buckling (M	lad)	3.55 III-K 3.42 in-k
Allowable shear force in w	eb (solid section)	idd)	975 lb
Allowable shear force in w	eb (perforated section)		399 lb
Unbraced length (Lu)			44.1 in
Torsional Propertie	25		
St. Venant torsion constan	t (J x 1000)		0.089 in⁴
warping constant (CW)	to neutral axis (Yo)		0.140 IN° -1 470 in
Distance between shear of	enter and web centerline	e (m)	0.859 in
Radii of gyration (Ro)		5 (11)	1.898 in
Forsional flexural constant	(Beta)		0.401
Code Approvals &	Performance Sta	ndards	
AISI S100-16 - North America • Effective properties incorpora • Gross properties are based of	an Specification for the D ate the strength increase from the cross section away f	esign of CFS Structor om the cold work of for rom the punchouts	u ral Members prming
AISI S240-15 - North America (Compliant to ASTM C955, bu	an Standard for Cold-For IBC replaced with AISI S2	med Steel Structural 00 in IBC 2015, AISI	Framing S240 in IBC 2017)
 Section A3 - Material - Chem Section A4 - Corrosion Prote Section A5 - Products - Third 	ical & mechanical requiren ction (Referencing ASTM A	nents (Referencing AS A653/A653M) identification	5TM A1003/A1003M)
 Section C - Installation - (Re 	erencing ASTM C1007)		
AISI S202-15 - Code of Stand	lard Practice for Cold-For	med Steel Structura	l Framing
Secular F3 - Delivery, Handl	ming and Storage of Material	5	

Product catego Product name:	S162 (1-5/8" Flange Structure 250S162-33 (33ksi, CP60) P 33mils (20ga)		e Structural Stu , CP60) P - Pu	d) nched	
		0011110 (20gu)	Color coding:	White
Geometric Pron	oertie	5		Color county.	WINC
Web depth	2,500	in			
Flange width	1.625	in	Punchout v	width	0.75 in
Stiffening lip	0.500	in	Punchout I	ength	4.00 in
Design thickness	0.0346	6 in	Min. steel t	hickness	0.0329 in
Yield strength, Fy	33 ksi		Fy with Co	ld-Work, Fya	33.0 ksi
Ultimate, Fu	45.0 k	si			
Gross Section P	Pronei	ties of	Full Sect	ion. Strong	Δχίς
Cross sectional area ((A)		i un occc	ion, otrong i	0.223 in^2
Member weight per fo	ot of ler	ngth			0.76 lb/ft
Moment of inertia (Ix)					0.235 in ⁴
Section modulus (Sx)	\sim				0.188 in ³ 1 027 in
Gross moment of iner	<) tia (Iv)				0.087 in ⁴
Gross radius of gyrati	on (Rý)				0.624 in
Effective Sectio	on Pro	perties	. Strona	Axis	
Effective Area (Ae)			,		0.172 in ²
Moment of inertia for	deflectio	n (Ix)			0.235 in ⁴
Section modulus (Sx)	mont (N	40)			0.180 in ³
Allowable bending mo	sed on i	/la) distortion	huckling (Ma	4)	3.55 IN-K 3.42 in-k
Allowable shear force	in web	(solid sec	tion)	-)	975 lb
Allowable shear force	in web	(perforate	d section)		399 lb
Unbraced length (Lu)					44.1 in
Torsional Prope	erties				
St. Venant torsion cor	nstant (J	x 1000)			0.089 in ⁴
Warping constant (Cw	V) Nontor to	noutral o			0.146 in ^o
Distance between she	enter to	er and we	b centerline (m)	0.859 in
Radii of gyration (Ro)					1.898 in
Torsional flexural con	stant (B	eta)			0.401
Code Approvals	5 & Pe	rforma	nce Stan	dards	
AISI S100-16 - North An • Effective properties inc • Gross properties are ba AISI S240-15 - North An (Compliant to ASTM C95 • Section A3 - Material - • Section A4 - Corrosion • Section A5 - Products - • Section C - Installation AISI S202-15 - Code of • Section F3 - Delivery, H ClarkDietrich's structur	nerican s orporate ased on t 55, but IB Chemica Protectio - Thickne - (Refere Standard Handling ral framin	Specificati the strengt the cross se Standard f C replaced I & mechar on (Referen ss, shapes encing AST I Practice and Storag ng comply	on for the Des h increase from ection away from or Cold-Forme with AISI S200C incal requirement cing ASTM A6 , tolerances, ide M C1007) for Cold-Form e of Materials with:	ign of CFS Structu in the cold work of fo m the punchouts id Steel Structural 0 in IBC 2015, AISI 3 ints (Referencing AS 53/A653M) entification ed Steel Structura	Iral Members rming Framing 5240 in IBC 2017) STM A1003/A1003M) I Framing

 Intertek CCRR-0206 SFIA Code Compliance Certification Program
 ICC-ES ESR-1166P

ICC-ES ESR-1166P - LABC and LARC Supplement

Demolition Waste Management (up to 2 points)

lotes for Allowable Stud Shear: Capacities are calculated per AISI S100-12.

For perforated sections, the standard ClarkDietrich oval knockout of 1-1/2" (3/4" for 2-1/2" & 3-1/2" webs) x 4" is used.

Complies with AISI S100-12 • IBC 2015 The technical content of this literature is effective 6/1/17 and supersedes all previous information.

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HRS/ST/FHA/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI Strap Ties

IPSO strong-Ti

	Fasteners⁴ (Total)		Allowa	able ASD Tension (lb.)	n Load			
Rafte	er/Stud/Joist Thick	ness	33 mil	43 mil	54 mil	Code Ref.		
(20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	(20 ga.)	(20 ga.) (18 ga.) (16 ga.)		(20 ga.) (18 ga.) (16		
) #10	(16) #10	(12) #10	1,945	1,950	1,950			
) #10	(18) #10	(16) #10	1,950	1,950	1,950			
) #10	(16) #10	(10) #10	1,765	2,025	2,025			
) #10	(20) #10	(12) #10	2,455	2,455	2,455			
#10	(8) #10	(8) #10	705	1,050	1,350			
) #10	(10) #10	(8) #10	885	1,315	1,350			
) #10	(12) #10	(12) #10	1,235	1,350	1,350			
) #10	(20) #10	(20) #10	1,350	1,350	1,350			
) #10	(36) #10	(30) #10	3,180	4,600	4,600			
) #10	(46) #10	(46) #10	4,595	4,600	4,600			
) #10	(42) #10	(42) #10	4,600	4,600	4,600	IBC, FL, LA		
) #10	(46) #10	(30) #10	5,795	5,795	5,795			
) #10	(46) #10	(30) #10	5,795	5,795	5,795			
) #10	(30) #10	(18) #10	3,535	3,760	3,760			
#10	(6) #10	(6) #10	530	790	1,600			
) #10	(10) #10	(10) #10	885	1,315	2,670			
) #10	(14) #10	(12) #10	1,235	1,840	2,710			
) #10	(26) #10	(22) #10	2,300	3,420	5,025			
) #10	(36) #10	(22) #10	3,180	4,735	5,025			
) #10	(40) #10	(22) #10	4,240	5,025	5,025			
) #10	(40) #10	(22) #10	5,025	5,025	5,025			
) #10	(30) #10	(22) #10	2,650	3,945	5,025			
) #10	(40) #10	(34) #10	3,710	5,025	5,025			
) #10	(54) #10	(46) #10	4,770	5,155	5,155	_		
) #10	(68) #10	(62) #10	5,820	6,420	6,650			
) #10	(72) #10	(64) #10	6,650	6,650	6,650			

4. See the currrent Fastening Systems catalog on strongtie.com for more information on Simpson Strong-Tie fasteners.

1-1/2" Furring / Hat Channel 150F125-33 33mil (20ga STR) **Geometric Properties** Depth: 1.500 in Design Thickness: 0.0346 in Yield stress, Fy: 33ksi Width: 1.250 in Min. steel thickness: 0.0329 in Ultimate, Fu: 45ksi Gross Section Properties of Full Section, Strong Axis 0.174 in² Cross sectional area (A) Member weight per foot of length 0.590 lb/ft Moment of inertia (lx) 0.056 in⁴ Radius of gyration (Rx) 0.566 in 0.093 in⁴ Gross moment of inerita (ly) Gross radius of gyration (Ry) 0.733 in **Effective Section Properties, Strong Axis** 0.056 in⁴ Moment of inertia for deflection (lx) 0.071 in³ Section modulus (Sx) 117.31 ft-lb Allowable bending moment (Ma) 474 lb Allowable shear force (Va)

Product Data & Ordering Information:

Coating: CP60 per ASTM C955 (G90 Available) Stock length: 10'-0" and 12'-0" long pieces

Packaging: 10 pieces per bundle - 440 pieces per pallet Packaging weight: 54 lbs/bundle

Dead Load		4 psf			6 psf			13 psf		
Hat Spacing o.c.	12"	16"	24"	12"	16"	24"	12"	16"	24	
Single Span	8'-6"	7'-8"	6'-9"	7'-5"	6'-9"	5'-10"	5'-9"	5'-2"	4'-6'	
Multiple Span	10'-6"	9'-6"	8'-4"	9'-2"	8'-4"	7'-3"	7'-1"	6'-5"	5'-7'	

Code Approvals & Performance Standards

- AISI S100-16 North American Specification for the Design of CFS Structural Members AISI S220-15 North American Standard for CFS Framing - Nonstructural Members
- (Compliant to ASTM C645, but IBC replaced with AISI S220 in IBC 2015) • Section A4 Material - Chemical & mechanical requirements (Referencing ASTM A1003/A1003M) Section A5 Corrosion Protection (Referencing ASTM A653/A653M)
- Section C Installation (Referencing ASTM C754) AISI S240-15 North American Standard for Cold-Formed Steel Structural Framing • (Compliant to ASTM C955, but IBC replaced with AISI S200 in IBC 2015, AISI S240 in IBC 2018) • Section A3 Material - Chemical & mechanical requirements (Referencing ASTM A1003/A1003M)
- Section A4 Corrosion Protection (Referencing ASTM A653/A653M) • Section C Installation - (Referencing ASTM C1007)
- SDS For ASTM A1003 Steel Framing Products For Interior Framing, Exterior Framing and Clips/Accessories

Sustainability Credits For more details and LEED letters contact Technical Services at 888-437-3244 or visit

- LEED v4.1 MR Credit: Environmental Product Declarations: EPD (1 point) Sourcing of Raw Materials (up
- to 2 points) Material Ingredients (1 point) Construction and Demolition Waste Management (up to 2 points) • LEED v4 MR Credit: Building Product Disclosure and Optimization: EPD (1 point) - Sourcing of Raw Materials (1 point) - Material Ingredients (1 point) - Construction and Demolition Waste Management (up to 2 points) - Innovation Credit (up to 2 points).

• Furring for walls & ceilings

Furring out masonry walls

Product Submittal Sheet

Technical Services: 888-437-3244 Engineering Services: 877-832-3206

Sales: 800-543-7140

clarkdietrich.com

Structural Punchout East Coast / Central punch spacing: Center of punchouts are 12" from lead end, then 24" o.c.

West Coast punch spacing: Center of punchouts are 24" from lead end, then 24" o.c. Center of tail end puchout not less than 12" from end of stud. If lateral bracing is required for head-of-wall deflection track and a punchout is not spaced 12" from the top of stud, use strapping and blocking in lieu of CRC or Spazzer Bar lateral bridging. If custom punchout patterns are required,

contact ClarkDietrich Sales or local plant for requests.

Sustainability Credits For more details and LEED letters contact Technical Services at 888-437-3244 or visit clarkdietrich.com/LEED. LEED v4.1 MR Credit: Environmental Product Declarations: EPD (1 point) - Sourcing of Raw Materials (up to 2 points) - Material Ingredients (1 point) - Construction and LEED v4 MR Credit: Building Product Disclosure and Optimization: EPD (1 point) - Sourcing of Raw Materials (1 point) - Material Ingredients (1 point) - Construction and Demolition Waste Management (up to 2 points) - Innovation Credit (up to 2 points).

CD-STRS © 12/22 ClarkDietrich

Product Submittal Sheet Technical Services: 888-437-3244, Engineering Services: 877-832-3206, Sales 800-543-7140

• SDS & Product Certification Information is available at www.clarkdietrich.com/SupportDocs

09.22.16 (Non-Structural Metal Framin

Used in framing applications:

1900 MARKET STREET ©2023 PHILADELPHIA, PA 19103

2023.03.30

SHOP DWGS FOR **EXTERIOR ARCADE**

PROJECT:

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787

DATE: PROJECT NO.:	03/30/20 23-00
REVISION	DA
GENERAL REV.	5/3/20

NOTES:

SPECIFICATIONS

AS NOTED SCALE:

SD-16.0

DRAWN BY: CHECKED BY:

IM PJ Company : Reflect Design + Build Group Designer : JEM Job Number : 23-0004

Model Name : Simply Capri Hip Truss

Wind Load Generator

Wind Generation Input

Wind Code	ASCE 7-16		Directionality Factor,K _d	0.85
Wind Speed, V	139	mph	Gust Stiffness	Rigid
Exposure Category	C		Ground Elevation Factor, K $_{\rm e}$	1
Base Elevation	0	ft		
Topographic Effect	cts			
Factor K ₁ 0	Factor K ₂ 0		Factor K ₃ 0	

Wind Generation Detail Results

Exposure Constant Alpha	9.5	K _{zt}	1	g
Exposure Constant, zg	900	h	4	ft
Gust Effect Factor, G	0.85	К _h	0.849	g
Windward C _p	0.8	q _h	0.036	ksf

Wind Generation Floor Geometry Results

			-			
Floor Level	Height (ft)	Kz	Width (Z) (ft)	Length (X) (ft)	Leeward Cp (Z)	Leeward Cp (X)
Diaphragm 1	13	0.849	0	0	0.2	0.2

Wind Generation Floor Force Results Floor Level Windward eeward Leeward 07

Floor Level	qz (kcf)	Windward	Leeward Prossure (7)	Leeward Brossure (X)	Force (Z)	Force (X)
	(K31)	(ksf)	(ksf)	(ksf)	(Kips)	(KIPS)
Diaphragm 1	0.036	0.024	0.006	0.006	0	0

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Wind Load Generator

Wind Gene	ration Inp	but	
Wind Code	ASCE 7-16		Dir
Wind Speed, V	139	mph	Gu
Exposure Category	С		Gro
Base Elevation	0	ft	
Topographic Effe	cts		
Factor K ₁ 0	Factor K ₂	0	Facto

Wind Generation Detail Results

Exposure Constant Alpha	9.5	K _{zt}
Exposure Constant, zg	900	h
Gust Effect Factor, G	0.85	К _h
Windward C _p	0.8	q _h

Wind Generation Floor Geometry Results

Floor Level	Height (ft)	Kz	Width (Z) (ft)	Length (X) (ft)	Leeward Cp (Z)	Leeward Cp (X)
Diaphragm 1	13	0.849	0	0	0.2	0.2
Wind Gen	eration I	Floor Forc	e Results			
Floor Level	qz (ksf)	Windward Pressure (ksf)	Leeward Pressure (Z) (ksf)	Leeward Pressure (X) (ksf)	Force (Z) (kips)	Force (X) (kips)

Floor Level	Height (ft)	Kz	Width (Z) (ft)	Length (X) (ft)	Leeward Cp (Z)	Leeward Cp (X)
Diaphragm 1	13	0.849	0	0	0.2	0.2
Wind Gene	ration	Floor Force	e Results			
Floor Level	qz (ksf)	Windward Pressure (ksf)	Leeward Pressure (Z) (ksf)	Leeward Pressure (X) (ksf)	Force (Z) (kips)	Force (X) (kips)
Diaphragm 1	0.036	0.024	0.006	0.006	0	0

RISA-3D Version 21

[Hip Truss trrying.r3d]

Page 1

RISA-3D Version 21

5/18/2023	
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Directionality Factor, K _d 0.85 Rigid ust Stiffness Ground Elevation Factor, K _e 1

ctor K ₃ **0**

1 g 4 ft **0.849** g 0.036 ksf

[Truss trying.r3d]

Page 1

DRAWN BY: CHECKED BY:

SCALE:

PJ

AS NOTED

SD-17.0

CALCULATIONS

NOTES:

130 RUBY RED PLACE FC, WINTER GARDEN, FL 34787 03/30/2023 DATE: PROJECT NO .: 23-0004 DATE REVISION GENERAL REV. 5/3/2023

PROJECT: SHOP DWGS FOR **EXTERIOR ARCADE**

2023.03.30

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